

Chapter 9

Strain localization modeling in coupled transient phenomena

9.1. Introduction

In many Civil Engineering problems (in a broad sense), the rupture of the foundation soil may be the relevant phenomenon for the global stability of a structure. For geomaterials, experimental observations show clearly localized ruptures [DES 96]. Some authors [KHO 06] refer also to diffuse mode of failure (which means homogeneous failure in laboratory tests). In the first case, the phenomenon is preceded by progressive strain localization within thin bands, leading finally to rupture lines. It is thus very important to be able to model numerically the post localization behaviour of geomaterials, for applications like the evaluation of bearing capacity, of slope stability or of damage zone around a tunnel... Furthermore, soils and rocks are porous materials, where the porous volume is filled with one or several fluids (water, gas, oil ...). The general behaviour of the medium depends not only on the skeleton response (solid phase) to a given loading path, but also on the interactions occurring between the different phases of the medium. Capillary effects, temperature variations, chemical reactions induce specific behaviours, which have to be modeled by multiphysical constitutive laws. It has been shown that these latter laws share a same structure [COL 05]. The numerical tools for the modeling of post failure problems have thus to be extended

to this multiphysic context, to deal with applications related to nuclear waste disposal, concrete behaviour under severe loading... Then, new questions arise concerning the interactions between localization process and physical process (like liquid diffusion for example). The answer to these new questions can only be given by experimental evidences. The section 2 of this paper will summarize the main observations on localization phenomenon, coming from the experimental results. The section 3 is related to the theoretical tools devoted to localization process. The section 4 is the description of the coupled second gradient model, which is the regularization technique used for the application described in section 5. Some conclusions end up the paper in section 6.

9.2. Experimental evidences

During the last 20 years, a lot of experimental work has been devoted to the study of strain localization in soils and rocks. These studies performed by Vardoulakis and co-workers ([VAR 78], [HAN 91]), Finno and co-workers ([FIN 96], [FIN 97]) and Desrues and co-workers ([DES 84], [DES 04]) among others have mainly focused on the purely mechanical problem (dry sample or globally drained at low strain rate) or on the hydromechanical problem (undrained or globally drained at fast strain rate). The study of the evolution of strain localization processes during the experiment needs for special techniques like stereophotogrammetry, 3D digital image correlation or X-ray micro tomography [LEN 07]. Thus, as in triaxial experiment (and more generally in axi-symmetric tests) the localization zone may remain more or less hidden inside the sample, most of the experimental campaigns on localization have been performed in biaxial apparatus, where this latter process is 'clearly' evidenced.

The main conclusions of the experimental works can be summarized as follow [DES 05]:

- Strain localization in shear band mode can be observed in most laboratory tests leading to rupture in geomaterials,
- Complex localization patterns may be the result of specific geometrical or loading conditions,
- Well marked stress peaks in stress-strain curves can be considered as the signature of an established shear band system over the specimens (Figure 1).

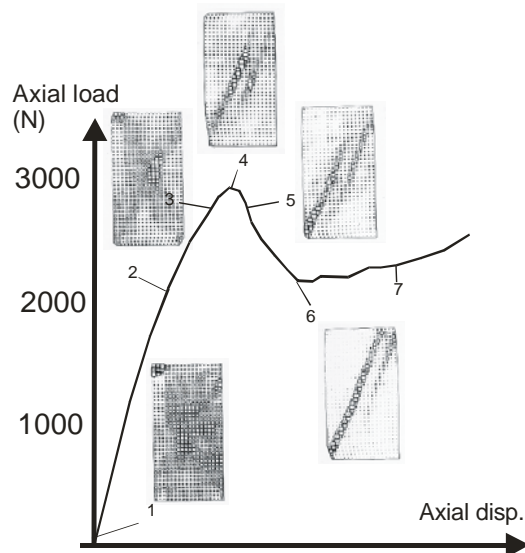


Figure 1. *Drained biaxial test on Hostun sand (After [MOK 99])*

Comparing to pure mechanical experiment (on dry sample), hydro-mechanical experiment introduces another physical phenomenon (i.e the fluid diffusion) that can influence the localization processes. This latter potential interaction depends of course on the (mechanical and diffusive) material properties, but also on the boundary conditions of the problem. For drained conditions (if the value of the permeability is sufficient low with respect of the velocity of the loading), the interactions are inexistent, as far as no overpressures are generated and the experimental observations are equivalent to those corresponding to dry conditions. For undrained conditions, overpressures are generated and some interactions exist between the two processes. It should be pointed out that, in the laboratory, the boundary conditions which can be prescribed correspond to globally undrained conditions and not to locally undrained ones. It means that, even in undrained conditions, some fluid flows can be created. The undrained experiments performed in Grenoble ([MOK 99], [ROG 98]) among others have shown some common features and have lead to the following observations:

- Plane strain undrained tests performed on loose and dense Hostun sand exhibit localization pattern.
- For dense (dilatant) sand specimen, localization is possible only when cavitation occurs in the pore fluid.

This last observation clearly evidences the interaction between the localization and the fluid diffusion, as far as the diffusion processes inhibits the localization

before the cavitation within the pore volumes. Such phenomenon should thus be addressed both theoretically and numerically in a robust modeling of the rupture.

Experimental studies of the strain localisation in other multiphysic context (unsaturated conditions, thermo-mechanical, chemo-mechanical ...) are rather poor and there is clearly a need for further investigations. It is indeed important to know whether the temperature or the suction influence the occurrence of localization and the size of the process zone.

9.3. Regularization techniques

The experimental results show that shear band formation can occur in geomaterials for many loading conditions. The constitutive laws devoted to the modeling of their behaviour have to be able to reproduce such phenomenon. The principal questions are thus the following: how could we know if the constitutive model can predict a strain localization mode and if this bifurcation occurs, how could we model it properly?

Following the previous works by [HAD 03], [HIL 58] and [MAN 66], Rice and co-workers ([RIC 76], [RUD 75]) have proposed the so-called Rice criterion, which analyses the stress state and investigates the possibility of a bifurcation by formation of a shear band in the stress and strain paths, for incrementally linear behaviour. This criterion reads:

$$\det(nCn) \leq 0 \quad [1]$$

In the case of a classical single-mechanism elastoplastic model, it has been proved that the previous criterion holds, provided C is chosen as the constitutive tensor corresponding to plastic loading [CHA 86].

These developments are valid for pure mechanical analysis. In a multiphysic context, Loret and co-workers [LOR 91] showed that for hydromechanical problems the condition of localization depends only on the drained properties of the medium. Nevertheless, it should be emphasized that in coupled problems much more complex localization pattern can be obtained, at least theoretically (figure 2; [VAR 96], assuming an incrementally linear constitutive equation).

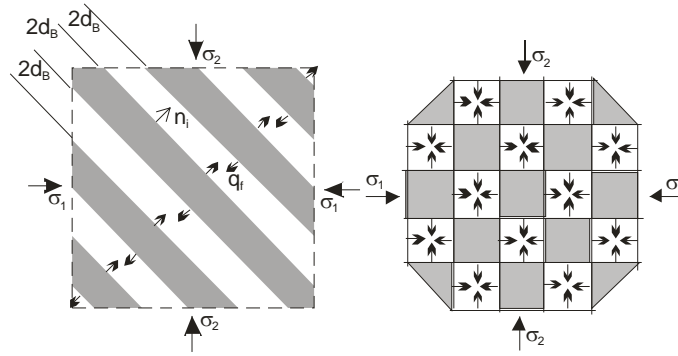


Figure 2. Shear banding mode in water-saturated sand ('Dark' strips contract and 'light' strips dilate) [VAR 96]

Rupture in localized mode is observed experimentally; theoretical tools permit to characterize it when the conditions of occurrence of such phenomenon are met. The further step is to define a correct and robust modeling of the strain localization leading finally to the rupture. However, it is well known that classical finite element models suffer of a pathological problem of mesh dependency for strain localisation modeling. This is due to properties of the underlying mathematical problem. In order to put in evidence this deficiency of classical tools, let us consider a very simple one-dimensional problem of a constant section bar under uniaxial tension (Figure 3), made of a material with a very simple constitutive law (Figure 4-a). This law may be the result of an elastic damage model or a strain softening elastoplastic model. It should be pointed out that the following discussion is also valid for a material layer under shear (replacing normal stress by shear stress).

When submitted to an axial displacement, the load-displacement curve remains linear up to the stress state reaches the yield stress f_y . After this point, the axial stress decreases but it has to remain homogeneous all along the bar according the static equilibrium condition. For each point of the bar, two options exist: either the softening loading or the elastic unloading. Let us define α as the ratio of the length L_s (where we have softening loading) over L . For $\alpha = 1$, the load will decrease linearly down to zero for $\Delta = L \varepsilon_r$. For a limit case ($\alpha = 0$), the load will decrease linearly down to zero at the origin without energy consumption (Figure 4-b). A priori, with classical numerical model, the length L_s is undetermined and, as far as the problem is ill-posed, we observe a mesh-dependency of the response of the model. For an algorithm finding the most localized solution, L_s will be equal to the size of the smallest element: if the number of element is increasing, the ratio α will tend to zero! The situation is even worse, when considering the complete initial boundary value problem (see [KOT 08]).

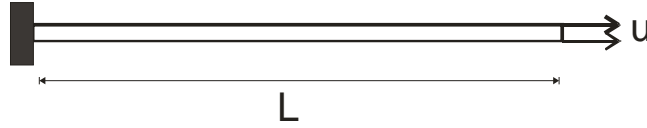


Figure 3. Constant section bar under uni-axial tension

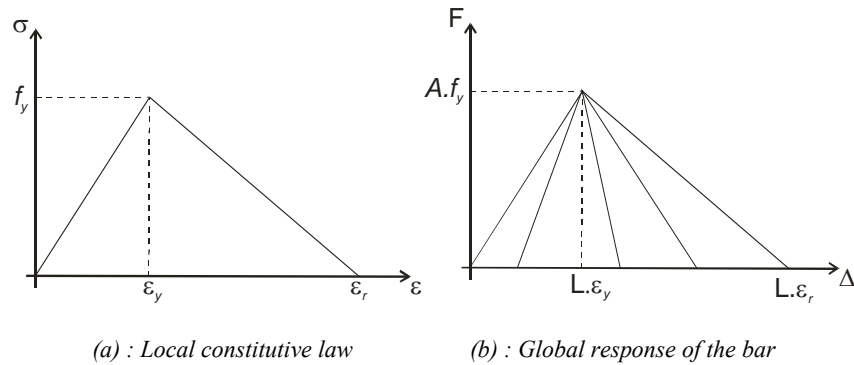


Figure 4. Local and global behaviour of the bar

This simple example evidences the need of an internal length scale for a correct modeling of the post peak behaviour. Several approaches have been proposed to introduce such internal length scale in the problem. We can distinguish two categories of regularization techniques:

- Enrichment of the constitutive law

The theory introduces an internal length scale at the level of the constitutive model. The advanced analysis of localization phenomena has shown that constitutive equations with internal length are necessary to properly model the experimental results involving some localized patterns: see for instance the pioneering works of Aifantis [AIF 84], which introduced gradient of internal variables, or the works of Bazant, Pijaudier-Cabot and co-workers ([BAZ 84], [PIJ 87]), which proposed a family of constitutive models derived from non local damage theory in which a non local internal variable is used instead of the local one. This latter theory has also generated the implicit gradient models [PEE 96], where the non local internal variable is an additional unknown and solution of the Helmholtz differential equation.

- Enrichment of the kinematics

The theories based on an enhancement of the kinematics itself can be traced back to the pioneering works of [TOU 62], [MIN 64] and [GER 73]. Here we consider the

materials with microstructure as defined by [MIN 64] and [GER 73]. Adding some mathematical constraints to the more general materials with microstructure yields a large set of models. Among all these models, the more ancient and famous one is the Cosserat model [COS 09]. In the following, we will mainly use the second gradient model developed in Grenoble ([CHA 98], [CHA 01]), but the obtained conclusions can be generalized to the other regularization techniques.

The extension of these regularization techniques to multiphasic materials and multi-physic problems leads to the question of interactions between the physical problems and the localization process. Loret and co-workers have been shown that diffusion equation (fluid or thermal) introduces an internal length scale [LOR 91]. Zhao, Schrefler and co-workers ([ZHA 99], [ZHA 01]) have studied the potential interaction between the different lengths. They showed that the interaction between the different phenomena can appear for a given range of parameters, nevertheless a robust mechanical model is still needed even for coupled problems.

9.4. Numerical modelling

Among the different regularization techniques, we propose in the following to use the second gradient model ([CHA 98][CHA 01]). The second grade model is extended to biphasic medium [COL 06] in order to show the possible interaction between the internal lengths introduced by the model and the diffusion process. In this model, the porous medium is considered as the superposition of several continua [COU 95]: the solid skeleton (grains assembly) and the fluid phases (water, air, oil...). Based on averaging theories [HAS 79a,79b], [LEW 00] proposed the governing equations for the full dynamic behaviour of a partially saturated porous medium. Hereafter these equations are restricted for quasi-static problem in unsaturated conditions, under Richard's assumptions (constant air pressure). This assumption may be irrelevant in the particular case of low and ultra low permeable porous media. For this study, isothermal condition and incompressible solid grains are assumed. The unknowns of the mechanical and the flow problems are respectively the displacements u_i and the pore water pressure p_w (possibly negative in unsaturated case). In the following developments, the balance equations are written in the current solid configuration denoted \mathcal{Q}^t (updated Lagrangian formulation).

9.4.1. Momentum balance equation

In the mixture balance of momentum equation, the interaction forces between fluid phases and grain skeleton cancels. In a weak form (virtual work principle), this equation reads for any kinematically admissible virtual displacement field:

$$\int_{\Omega^t} \sigma_{ij}^t \varepsilon_{ij}^* d\Omega^t = \int_{\Omega^t} \left(\rho_s (1 - \phi^t) + S_{r,w}^t \rho_w^t \phi^t \right) g_i u_i^* d\Omega^t + \int_{\Gamma_\sigma^t} \bar{t}_i^t u_i^* d\Gamma^t \quad [2]$$

where $\varepsilon_{ij}^* = 0.5 \left(\left(\frac{\partial u_i^*}{\partial x_j^t} \right) + \left(\frac{\partial u_j^*}{\partial x_i^t} \right) \right)$ is the kinematically admissible virtual strain field, ϕ^t is the porosity defined as $\phi^t = \Omega^{v,t} / \Omega^t$ where Ω^t is the current volume of a given mass of skeleton and $\Omega^{v,t}$ the corresponding porous volume, ρ_s is the solid grain density, $S_{r,w}^t$ is the water relative saturation, ρ_w^t is the water density, g_i is the gravity acceleration and Γ_σ^t is the part of the boundary where tractions \bar{t}_i^t are known.

The total stress σ_{ij}^t is defined as a function of the kinematics. Here we assume first that the Bishop's definition of effective stress holds [NUT 08]:

$$\sigma_{ij}^t = \sigma_{ij}^t - S_{r,w}^t p_w^t \delta_{ij} \quad [3]$$

with σ_{ij}^t the effective stress, p_w^t the pore water pressure and δ_{ij} the Kronecker symbol.

9.4.3. Mass balance equation

The water mass balance equation reads in a weak form:

$$\int_{\Omega^t} \left(\dot{M}^t p_w^* - m_i^t \frac{\partial p_w^*}{\partial x_i^t} \right) d\Omega^t = \int_{\Omega^t} Q^t p_w^* d\Omega^t - \int_{\Gamma_q^t} \bar{q}^t p_w^* d\Gamma^t \quad [4]$$

where p_w^* is the virtual pore water pressure field, Q^t is a sink term and Γ_q^t is the part of the boundary where the input water mass per unit area \bar{q}^t is prescribed. \dot{M}^t and m_i^t are respectively the mass of the water inside the current configuration of the skeleton Ω^t and the mass flow. They are defined hereafter respectively in Equation [7] and Equation [5].

Water mass balance equation (Equation 4) has to hold for any time t , the virtual quantities in this equation being dependant on the history of boundary conditions and on time t .

The mass flow m_i^t is defined as follows:

$$m_i^t = -\rho_w^t \frac{\kappa k_{r,w}^t}{\mu_w} \left(\frac{\partial p_w^t}{\partial x_i^t} + \rho_w^t g_i \right) \quad [5]$$

where κ is the intrinsic permeability, $k_{r,w}^t$ is water relative permeability and μ_w is the water dynamic viscosity.

The compressible fluid is assumed to respect the following relationship [LEW 00]. This predicts an increase of water density as a function of the pore water pressure, defining χ_w as the water bulk modulus:

$$\dot{\rho}_w^t = \frac{\rho_w^t}{\chi_w} \dot{p}_w^t \quad [6]$$

If the grains are assumed to be incompressible (which means ρ_s is constant), the time derivative of the water mass is obtained directly by using Equation [6] and mass balance equation for the solid phase. This yields for a unit mixture volume:

$$\dot{M}^t = \rho_w^t \left[\frac{\dot{p}^t}{\chi_w} S_{r,w}^t \phi^t + \dot{S}_{r,w}^t \phi^t + S_{r,w}^t \frac{\dot{\Omega}^t}{\Omega^t} \right] \quad [7]$$

9.4.3. Local second gradient model for monophasic medium

Let us recall first the governing equations of a monophasic medium with microstructure. This kind of models can be traced back to the pioneering works of the Cosserat brothers [COS 09], [MIN 64] and [GER 73]. In the framework of microstructure continuum theory, a microkinematic gradient field v_{ij} is introduced to describe strain and rotation at the microscale. With respect to classical continuum mechanics, additional terms are then added in the internal virtual work of a given body [GER 73]. The following expression holds for any virtual quantities:

$$W_i^* = \int_{\Omega^t} \left(\sigma_{ij}^t \frac{\partial u_i^*}{\partial x_j^t} + \tau_{ij}^t (v_{ij}^* - F_{ij}^*) + \Sigma_{ijk}^t h_{ijk}^* \right) d\Omega^t \quad [8]$$

where F_{ij}^* is the virtual macrodeformation gradient, v_{ij}^* is the virtual microkinematic gradient, τ_{ij}^t called microstress is an additive stress associated to the microstructure, h_{ijk}^* is the virtual microsecond gradient and Σ_{ijk}^t is the double stress dual of h_{ijk}^* .

In the local second gradient model used in the following, an assumption is added: the micro kinematic gradient is equal to the macro-deformation gradient

$$v_{ij} = F_{ij} \quad [9]$$

As a consequence: $v_{ij}^* = F_{ij}^*$. Finally, the principle of virtual work can be rewritten as follows:

$$W_i^* = \int_{\Omega'} \left(\sigma'_{ij} \frac{\partial u_i^*}{\partial x'_j} + \Sigma'_{ijk} \frac{\partial^2 u_i^*}{\partial x'_j \partial x'_k} \right) d\Omega' \quad [10]$$

Assuming that the boundary Ω' is regular, the external virtual work W_e^* can be defined as follows:

$$W_e^* = \int_{\Omega'} \rho g_i u_i^* d\Omega' + \int_{\Gamma'_\sigma} \left(\bar{t}_i u_i^* + \bar{T}_i D u_i^* \right) d\Gamma' \quad [11]$$

where \bar{t}_i is the external (classical) forces per unit area and \bar{T}_i an additional external (double) force per unit area, both applied on a part Γ'_σ of the boundary of Ω' . D denotes the normal derivative of any quantity q , ($Dq = (\partial q / \partial x_k) n_k$). To the authors' knowledge, the additional boundary condition (on \bar{T}_i) allows to produce solutions with boundary layers.

One of the advantages of local second gradient is the fact that constitutive equations remain local and therefore it is straightforward to formulate a second gradient extension of any classical models. The previous virtual work equation of second gradient models can of course be used in a finite element code. However, this equation needs the use of C^1 functions for the displacement field as second derivatives of the displacement are involved. In order to avoid such functions in the virtual work principle [10], the equalities between v_{ij}^* and F_{ij}^* and between v_{ij} and F_{ij} are introduced through a field of Lagrange multipliers λ_{ij} related to a weak form of the constraint [9] [CHA 98]:

$$\int_{\Omega'} \left(\sigma'_{ij} \frac{\partial u_i^*}{\partial x'_j} + \Sigma'_{ijk} \frac{\partial v_{ij}^*}{\partial x'_k} \right) d\Omega' - \int_{\Omega'} \lambda'_{ij} \left(\frac{\partial u_i^*}{\partial x'_j} - v_{ij}^* \right) d\Omega' - W_e^* = 0 \quad [12]$$

$$\int_{\Omega'} \lambda'_{ij} \left(\frac{\partial u_i^*}{\partial x'_j} - v_{ij}^* \right) d\Omega' = 0 \quad [13]$$

9.4.4. Local second gradient coupled model

Starting from Equations [2] and [4] of the coupled problem in classical poromechanics, microstructure effects can be introduced in the momentum balance equation by adding microkinematic gradient terms, under the previous assumptions ($v_{ij}^* = F_{ij}^*$, $v_{ij} = F_{ij}$). Assuming that pore fluid has no influence at the microstructure level, microkinematic gradient are not generated by pore pressure variations. This latter hypothesis follows the ideas of Ehlers [EHL 98] concerning a Cosserat model for biphasic medium. Cosserat model [COS 09] is in fact a particular case of microstructured medium.

According to the previous assumptions, we have the following governing equations:

$$\int_{\Omega'} \left(\sigma'_{ij} \frac{\partial u'_i}{\partial x'_j} + \Sigma'_{ijk} \frac{\partial^2 u'_i}{\partial x'_j \partial x'_k} \right) d\Omega' = \int_{\Omega'} \rho^{mix,t} g'_i u'_i d\Omega' + \int_{\Gamma'_e} \left(\bar{t}'_i u'_i + \bar{T}'_i D u'_i \right) d\Gamma' \quad [14]$$

$$\int_{\Omega'} \left(M' p'_w - m'_i \frac{\partial p'_w}{\partial x'_i} \right) d\Omega' = \int_{\Omega'} Q' p'_w d\Omega' - \int_{\Gamma'_q} \bar{q}' p'_w d\Gamma' \quad [15]$$

where $\rho^{mix,t}$ is the mass density of the mixture ($\rho^{mix,t} = \rho_s (1 - \phi) + S_r^{w,t} \rho^{w,t} \cdot \phi$). Introducing Lagrange multiplier fields (like in the previous section), the governing equations of the local second gradient coupled problem are then the following:

$$\int_{\Omega'} \left(\sigma'_{ij} \frac{\partial u'_i}{\partial x'_j} + \Sigma'_{ijk} \frac{\partial v'_{ij}}{\partial x'_k} \right) d\Omega' - \int_{\Omega'} \lambda'_{ij} \left(\frac{\partial u'_i}{\partial x'_j} - v'_{ij} \right) d\Omega' - W_e^* = 0 \quad [16]$$

$$\int_{\Omega'} \lambda'_{ij} \left(\frac{\partial u'_i}{\partial x'_j} - v'_{ij} \right) d\Omega' = 0 \quad [17]$$

$$\int_{\Omega'} \left(M' p'_w - m'_i \frac{\partial p'_w}{\partial x'_i} \right) d\Omega' = \int_{\Omega'} Q' p'_w d\Omega' - \int_{\Gamma'_q} \bar{q}' p'_w d\Gamma' \quad [18]$$

In the present work, the stress tensor σ_{ij} in Equation [16] is a total stress defined according to Bishop's postulate [3], on the contrary the double stress Σ_{ijk} has no link with the pore pressure.

Equations [16]–[18] have to hold for any time t , the virtual quantities in these equations being dependant on the history of boundary conditions and then on time t . Moreover the constitutive equations also have to hold for any time t .

9.4.5. Local second gradient model in a multiphysic context

The procedure to extend local second gradient model to other multiphysical context is more or less the same: additional balance equations appear to model the other processes. The main issue is not a numerical one but rather a physical one. What are the possible interactions between the second gradient model and the thermal diffusion, the suction or the chemical reaction? These questions should be first addressed by the experimental campaign. Concerning the internal length introduced by the second gradient model, it comes as the ratio of two constitutive moduli (the one related to the second gradient constitutive law and the one related to the classical law). Considering that the classical constitutive moduli are influenced by the different processes (chemical, thermal, suction ...), the conclusions should be that the internal length scale should be modified by these latter processes (under the condition that the second gradient law is not influenced by the processes). This should be again confirmed by some experimental evidences!

9.5. Applications

The following example exhibits the interest of using the theoretical tools presented but points out also their limitations. We propose to study a typical problem of nuclear waste disposal: the excavation of a storage gallery. It is proposed to model the excavation process with a very simple strain-softening constitutive law able to reproduce the main phenomena appearing during tunneling: the progressive decrease of material strength during loading and the strain localization. This coupled modeling is a benchmark exercise proposed by the GDR-Momas and organized by EDF-CEA [CHA 05].

9.5.1. Constitutive equation and localization study

The proposed constitutive law is an elasto-plastic strain-softening model. The yield criterion is a Drucker-Prager model given by the following equation:

$$F \equiv \sqrt{\frac{3}{2}} II_{\sigma^*} + m \left(I_{\sigma} - \frac{3c}{\tan \phi} \right) = 0 \quad [19]$$

where II_{σ^*} is the second deviatoric stress invariant, I_{σ} is the first stress invariant, ϕ is the friction angle, parameter m is a function of ϕ : $m = 2 \sin(\phi)/(3 - \sin(\phi))$, the cohesion $c = c_0 f(\gamma^p)$ is the softening variable, c_0 is the initial cohesion and γ^p is the equivalent deviatoric plastic deformation.

For this single mechanism elastoplastic law, the Rice criterion can be transformed in a fourth order equation in $\tan(\theta) = z$, with θ being the angle between n and the x -axis, and a_i being real parameters depending on the constitutive moduli:

$$\left(a_1 z^4 + a_2 z^3 + a_3 z^2 + a_4 z + a_5 \right) = 0 \quad [20]$$

At the beginning of loading, this equation does not have any real solution. After some load steps, the first real solution is a double one [WAN 93], which then gives two bifurcation directions.

In order to emphasize this theoretical concept with the proposed constitutive law, the homogeneous response of a drained biaxial experiment is first studied to observe when the Rice condition is met (Figure 5). Moreover, different dilatancy angles are considered to study the effect of this parameter on the response of the material. For all the considered values, the Rice criterion is met during the loading path.

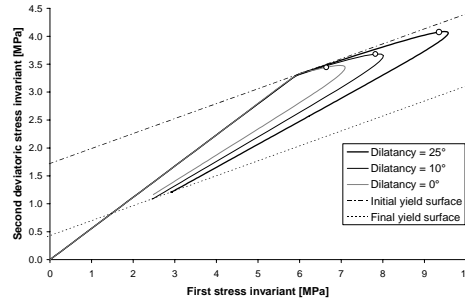


Figure 5. Drained biaxial test response of the constitutive law for several dilatancy angles

Figure 6 presents the evolution of the Rice criterion as a function of the loading history. When the behaviour is elastic, the criterion exhibits only one minimum and is always positive. The behaviour becoming elastoplastic, the criterion exhibits two extrema but remains positive up to a loading level, for which two symmetric

directions of bifurcation are predicted. The loading path going on, a fan of directions exists, where the Rice criterion is met.

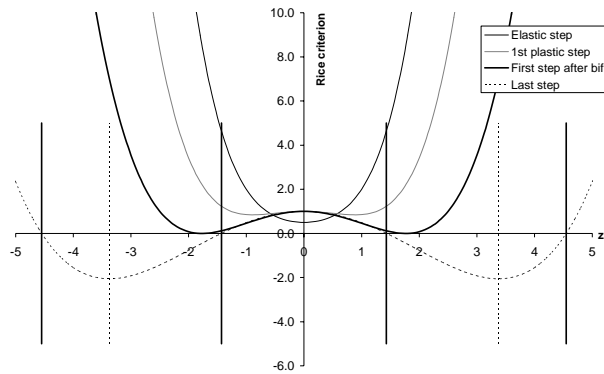


Figure 6. Rice criterion (as a function of z) for several loading steps

The numerical modelling of a perfect sample remains homogeneous even if the Rice criterion is met. In the reality, geomaterial samples have some heterogeneity, leading to localization processes. In order to reproduce numerically these processes, it is possible to introduce some defect (soft element, parasite force ...) in order to force the occurrence of localization (Figure 7). However, even in a perfect sample, it is possible to find localized solutions, using special techniques.

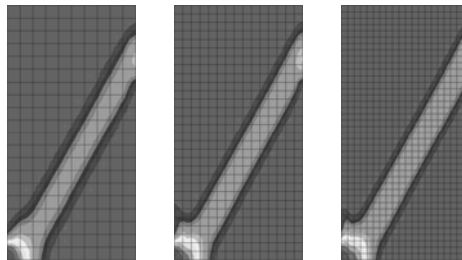


Figure 7. Localized solution for non-perfect sample for different mesh size (dilatancy=25°)

In a coupled context, the interactions between the mechanical behaviour and the fluid diffusion can modify the conclusions of the results obtained for drained biaxial tests. Let us now consider undrained biaxial tests in order to put in evidence the coupling effects.

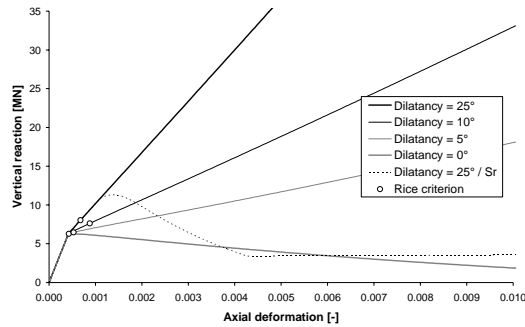
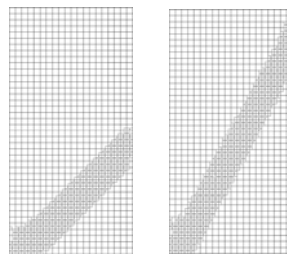


Figure 8. Load vs axial strain for undrained biaxial tests

The results (Figure 8) show that for highly dilatant materials the Rice criterion is never met (in this first modelling it is assumed that the medium remains saturated even for negative pore pressure). This is in agreement with the experimental observations. When the dilatancy is lower (from 10° to 0°), the Rice criterion is met and localization is possible. In order to put in evidence the influence of the unsaturated conditions, a case with a dilatancy of 25° and a retention curve is performed: it is shown that the Rice criterion is met when the sample becomes unsaturated!

Introducing a defect in the structure, it is possible to obtain localized solutions when the Rice criterion is met. Figure 9 presents the plastic points in the localized solution for the case of a null dilatancy angle and when the desaturation is taken into account. It is important to underline the fact that no localization is possible when the dilatancy is equal to 25° and the sample remains saturated.



(a) Dilatancy = 0° (b) Dilatancy = 25° and $S_r(s)$

Figure 9. Plastic loading points during undrained biaxial tests

9.5.3. Model predictions for the excavation problem

A cylindrical unsupported cavity of 3 m diameter is located in a homogeneous low permeability formation. The initial state of stress and pore pressure is the following:

$$\begin{aligned}\sigma'_{xx} &= \sigma'_{zz} = -7.74 \text{ MPa} \\ \sigma'_{yy} &= -11.64 \text{ MPa} \\ p_w &= 4.7 \text{ MPa}\end{aligned}$$

The excavation process is modeled by decreasing both radial total stress and the pore pressure at the cavity wall. The excavation duration T is equal to 1.5 Ms (about 17 days) and the final modeling time is 300 Ms (about 9.5 years).

$$\text{at } r = 3\text{m} \begin{cases} 0 \leq t \leq T \\ \sigma_{xx} = \sigma'_{xx} - bp_w = -11.5 \left(1 - \frac{t}{T}\right) \text{ MPa} \\ \sigma_{yy} = \sigma'_{yy} - bp_w = -15.4 \left(1 - \frac{t}{T}\right) \text{ MPa} \\ p_w = 4.7 \left(1 - \frac{t}{T}\right) \text{ MPa} \\ t > T \\ \sigma_{xx} = \sigma_{yy} = p_w = 0 \end{cases} \text{ and at } r = \infty \begin{cases} \sigma_{xx} = -11.5 \text{ MPa} \\ \sigma_{yy} = -15.4 \text{ MPa} \\ p_w = 4.7 \text{ MPa} \end{cases}$$

During the excavation, the behaviour of the material becomes plastic near the tunnel and permanent strains are created. The yield criterion is first met in the direction where the orthoradial stress is the major principal stress. Dilatancy effects are evidenced on Figure 10-a, where the pore pressure becomes negative at the end of the tunnelling. Then, during the remaining modelling time, the pore pressure increases progressively. Figure 10-b shows the radial displacement curve: during the tunnelling phase, cavity convergence remains around 1,5 cm. Most of the convergence appears later and reaches 22,6 cm. Indeed, negative pore pressure allows a additional cohesive strength; this effect is maximum after 1,5 Ms and decreases progressively still 300 Ms.

Shear band localization is also influenced by the hydro-mechanical coupling. Indeed, at the end of excavation, there is no clear localization patterning even if the Rice's criterion is verified. After that phase, pore pressure increases progressively and a patterning is gradually created. Figure 11-a presents a map of the equivalent strains, where the different shear bands clearly appear. The Vilotte's indicator is

shown in Figure 11-b; this indicator evidences the strain activity of the shear band. One can see here that only the external shear band is active at the end of the simulation and that a chip is finally created.

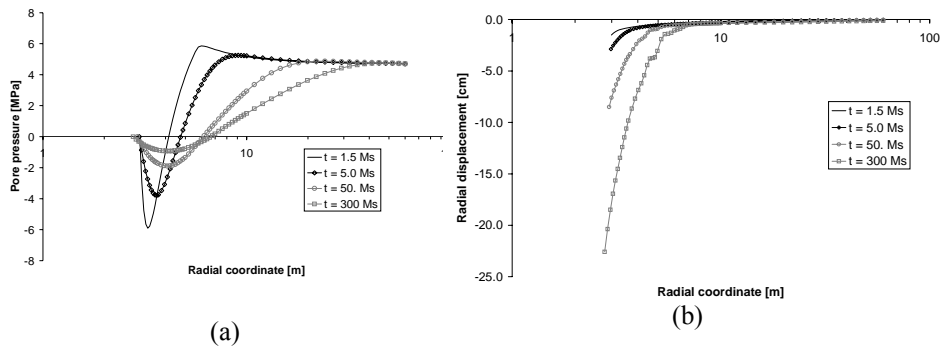


Figure 10. Pore pressure and displacement curve during coupled excavation

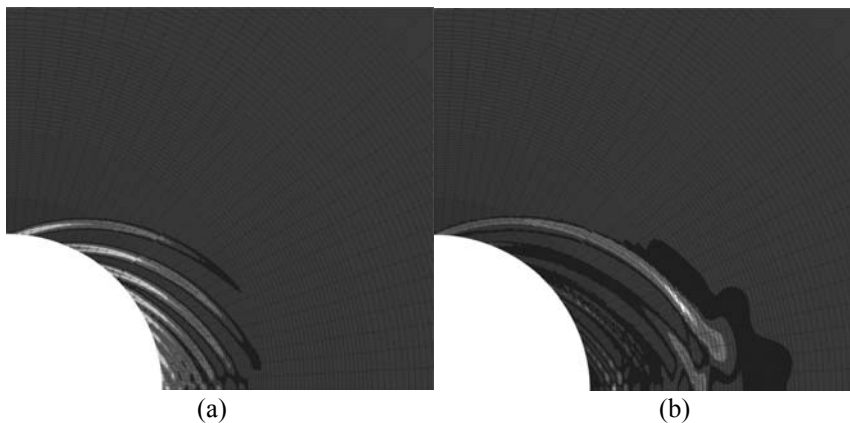


Figure 11. Equivalent strain and Vilotte's indicator for active shear band ($t = 300$ Ms) – Classical medium

These results (Figure 11) have been obtained with classical finite elements and are thus mesh dependent. We propose now to perform the same modelling with coupled second gradient model. We see (Figure 12) that the results are now independent of the mesh size. Regularization techniques are a solution for this problem but it should be pointed out that these theories do not restore the uniqueness of the solution.

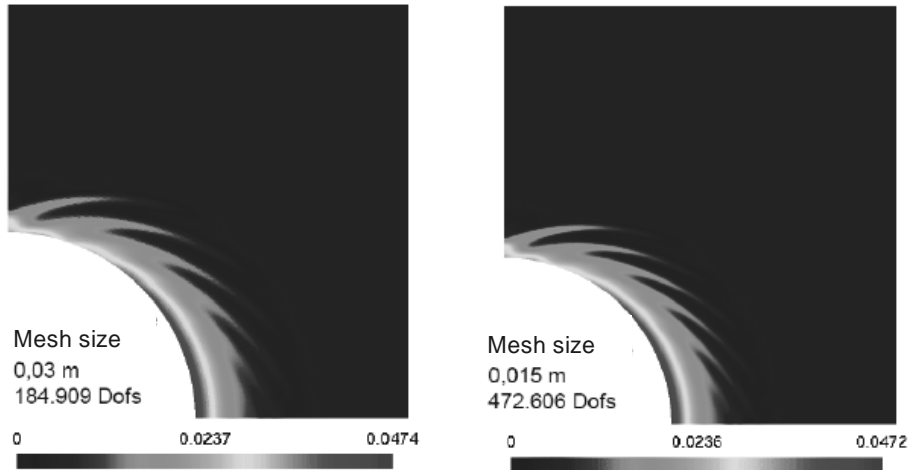


Figure 12. Equivalent strain for two mesh sizes with second gradient dilation model [FER 09].

9.6. Conclusions

Rupture in geomaterials is often preceded by a localization of the deformations within thin bands. The strain localization is thus an important process, which has been studied both experimentally and theoretically. The developments of geomechanics in the field of coupled multiphysic processes impose the study of the strain localization to these new conditions. Interactions between the different processes can indeed occur.

The numerical modelling of shear bands with classical finite element suffers of a mesh dependency problem. An internal length scale has to be introduced in the problem. Among the different regularization techniques, we propose a second gradient coupled model for an application to gallery excavation. It has been shown that the model regularizes the solution but does not restore the uniqueness (see [SIE 09]). The extension of such theories to other multiphysic context is more an experimental problem than a numerical one. Experiments still have to exhibit the influence of temperature, suction or chemical concentration on the occurrence and the thickness of the strain localization!

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9.7. References

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