

**Modeling tracer injection and well-aquifer interactions:
a new mathematical and numerical approach**

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Abstract

A new mathematical and numerical approach is presented to model solute exchange between a well and the surrounding aquifer for the interpretation of field tracer tests. Based on water and tracer mass balance equations integrated over the volume of water in the well, the approach allows for finite volumes of tracer fluid and water flush. It deals with tracer mixing and capturing in the well bore, local distortion of the flow field around the well and possible tracer back-migration into the well. A numerical solution, implemented in the three-dimensional finite-element groundwater flow and transport simulator SUFT3D, is proposed that allows for modeling non-uniform distributions of tracer mass fluxes along the well screens related to variations in aquifer hydraulic conductivity. Showing its ability to reproduce concentration evolutions monitored in a well during field tracer experiments, considering various injection conditions, validates the approach.

AGU Index terms: 1831 Hydrology: Groundwater quality, 1832 Hydrology: Groundwater transport

Keywords: tracer techniques, tracer injection, groundwater transport modeling

1 Introduction

Tracer experiments are frequently performed to identify aquifer transport processes and to quantify the governing hydrodispersive parameters. Many physical factors, related to experimental conditions and well-aquifer interactions, may lead to a tracer input function that departs strongly from commonly assumed instantaneous or step injection profiles. If not explicitly considered, this can lead to severe misinterpretation of the results. In particular, *Gelhar et al.* [1992] mention that, in terms of dispersivity assessment, a clear definition and control of the tracer input function are important factors for classifying the tracer experiment

as reliable. Nevertheless, little attention is usually given to this experimental step or to the accuracy of mathematical or numerical representations of the tracer injection.

A new physically-based approach is developed in order to accurately model tracer injection in a well. It is able to account for finite volumes and flow rates of tracer fluid, untraced water flush, mixing and capturing in the well bore, and complex well-aquifer interactions. The possibility of accounting for the influence of aquifer heterogeneity close to the injection well is also discussed. The implementation of the numerical scheme in a groundwater flow and transport numerical simulator (here, in the SUFT3D code) is described. For validation of these developments, the model is used to fit concentration evolutions monitored in a well during field tracer experiments.

2 Main factors influencing the injection process

When the tracer is injected in a well, its actual input function in the aquifer may be influenced by several factors. First of all, the duration and flow rates associated with the tracer injection and the water flush can play an important role [Guvanasen and Guvanasen, 1987; Brouyère and Rentier, 1997]. However, the experimenter can control these factors. Other key factors, related to well configuration and well-aquifer interactions, are not directly controlled and are often disregarded.

When the volume V_{inj} (L^3) of tracer is injected, a dilution occurs with the volume of water V_w (L^3) in the well. In spite of the injection of a water flush, a quantity of tracer may remain temporarily captured in the well bore. These so-called mixing effects that potentially result in lower recovery peaks at observation or pumping wells [Novakowski, 1992a; Moench, 1989] are usually quantified with a non-dimensional mixing factor $V_{inj}^* = V_{inj} / V_w$.

When injection operations are completed, the remaining quantity of tracer in the well is progressively released to the aquifer due to the natural-gradient transit flow rate crossing well screens. This flow rate is affected by well bore skin effects, which are often considered by means of a non-dimensional lumping distortion coefficient α_w , expressing the ratio between the actual water flow rate crossing the well section orthogonal to the main flow direction and the theoretical flow rate that would transit across the same section if the well was not present [Drost *et al.*, 1968, Hall, 1996, among others]:

$$\alpha_w = \frac{Q_t^0}{2r_w e_{scr} |\underline{v}_D|} \quad (1)$$

Q_t^0 is the flow rate crossing the well screens (L^3T^{-1}) in natural flow conditions, r_w is the screen casing radius (L), e_{scr} is the screen-length (L), and $|\underline{v}_D|$ is the mean Darcy flux (LT^{-1}) that would prevail close to the well in the absence of flow distortion.

Finally, due to the heterogeneity of the aquifer, the distribution of water fluxes along the screens may lead to a non-uniform tracer spreading in the aquifer, in the vicinity of the injection well.

3 Mathematical model

3.1 Existing approaches

Guvanasen and Guvanasen [1987] have developed a semi-analytical solution that deals with a finite volume of tracer fluid and water flush but does not consider well-bore mixing and skin effects. The analytical solutions of *Novakowski* [1992a] allow for tracer column displacements, considering mixing effects in reservoirs connected to column inlet and outlet, but are limited to the one-dimensional case. The analytical solution of *Moench* [1989], applied

to radially converging tracer tests, considers well-bore mixing effects in the injection and extraction wells but a Dirichlet-type boundary condition is used to link concentrations in the well bore and in the surrounding aquifer. *Novakowski* [1992b] determined that, from a physical point of view, the third-type boundary conditions better represent experimental data. Aside from proposing a correction (see *Moench*, 1996) to the solution proposed by *Moench* [1989], *Zlotnik and Logan* [1996] discuss concentration-based and mass flux-based boundary conditions applied at the interface between the injection well and the aquifer, with the assumption that the tracer injection rate is small and, therefore, does not alter the velocity distribution in the aquifer around the injection well. Thus, actual injection conditions dealing with a finite volume of tracer and water cannot be considered. Several approaches have also been proposed for modeling wells using one-dimensional highly conductive finite elements [*Sudicky et al.*, 1995, *Wu et al.*, 1996, among others]. Being one-dimensional, these elements do not consider the influence of skin effects on the flow field pattern close to the well.

It appears that none of the existing approaches is able to deal with the full complexity associated with the tracer injection process and actual field conditions. In what follows, a new mathematical model is presented that allows for the tracer injection operations actually encountered in the field to be modeled.

3.2 Mass balance equations applied to water and tracer in the well bore

The model is based on mass balance equations applied to water and solute, integrated over the volume of water V_w in the well bore (Figure 1). Flow rate terms account for different possible exchanges between the well and its environment: the injection flow rate Q_{in} , the transit flow rate Q_t entering the well through the screens and the flow rate Q_{out} leaving the well through the screens. Tracer concentrations associated with these flow rates and in the injection well

are respectively C_{in} , C_t , C_{out} and C_w . All these terms may vary with time. The well radius is r_w and the length of the water column in the well is h_w . If density effects, due to the presence of a solute in the water, are neglected the mass balance equation applied to water within the well can be written as follows:

$$\frac{\partial V_w(t)}{\partial t} = Q_{in}(t) + Q_t^{in}(t) - Q_{out}(t) \quad (2)$$

where $V_w = \pi r_w^2 h_w$ is the volume of water in the well at time t , the superscript 'in' appearing in Q_t^{in} reflects a dynamic link with Q_{in} (see section 3.3).

The hypothesis of perfect mixing of the tracer and water in the well bore is assumed. This may be facilitated by pumping water from the bottom part of the well and discharging it at the upper part of the water column. Based on that, $C_w(t)$ represents the mean concentration in the well at time t . It is also assumed that the tracer concentration C_{out} is equal to the concentration C_w in the well.

Finally, the mass balance equation applied to the tracer within the well is given by:

$$\frac{\partial M(t)}{\partial t} = \frac{\partial}{\partial t}(V_w C_w) = \pi r_w^2 \left(C_w \frac{\partial h_w}{\partial t} + h_w \frac{\partial C_w}{\partial t} \right) = Q_{in} C_{in} + Q_t^{in} C_t - Q_{out} C_w \quad (3)$$

3.3 Evaluation of the transit flow rate and concentration

The transit flow rate depends on the injection rate. If Q_{in} is low, Q_t^{in} is close to natural flow conditions (Figure 2a). As Q_{in} is increased, it progressively diminishes Q_t^{in} (Figure 2b). For a critical value $Q_{in} = Q_{cr}$, Q_t^{in} is exactly cancelled (Figure 2c). Above the critical injection rate ($Q_{in} > Q_{cr}$), all water leaving the well through the screens is injected water, this water being

spread in all directions around the injection well (Figure 2d). To account for the continuous variation of Q_t^{in} according to injection conditions, an analytical formulation has been deduced from the potential theory presented by *Bidaux and Tsang* [1991]. It can be shown that the resulting equation is given by:

$$Q_t^{in} = 2r_w e_{scr} \alpha_w |v_D| \sin(\arccos Q_{in}^*) - \frac{Q_{in}}{2\pi} (2 \arccos Q_{in}^*) \quad (4)$$

where $Q_{in}^* = Q_{in}/Q_{cr}$, with $Q_{cr} = 2\pi r_w e_{scr} \alpha_w |v_D|$ the critical injection rate.

If $Q_{in}^* = 0$, Equation (4) simplifies to the expected expression for the natural transit flow rate:

$$Q_t = Q_t^0 = \alpha_w S_w |v_D| = 2e_{scr} r_w \alpha_w |v_D| \quad (5)$$

Equation (4) assumes that locally all fluxes reach equilibrium almost instantaneously (i.e., $\partial h_w / \partial t = 0$). This assumption is not valid if the injection rate is very high or if the aquifer hydraulic conductivity or well bore skin permeability is low. However, in this situation, it is likely that the transit flow rate would be canceled.

To evaluate the natural transit flow rate (Equation 5), Darcy velocities prevailing in the aquifer close to the injection well have to be estimated without the influence of any source/sink term applied in the injection well. In a homogeneous aquifer, this can be based on an estimation of the local hydraulic conductivity and gradient. If pure radially converging flow conditions prevail, the following expression can be used:

$$|v_D| = \frac{Q_P}{2\pi d e_{aq}} \quad (6)$$

Q_p is the extracted flow rate at the pumping well, d is the distance between pumping and injection wells, and e_{aq} is the mean saturated thickness of the aquifer.

During injection and water flush, the transit mass flux ($f_{Mt} = Q_t^m C_t$) is set to zero. Indeed, it can be expected that either $Q_{in} > Q_{cr}$, in which case Q_t is equal to zero, or Q_{in} is low and the transit flux does not carry tracer (i.e., $C_t = 0$). When Q_{in} is set to zero (tracer injection or water flush completed), it is assumed that $C_t = C$, the latter being the mean concentration in the aquifer around the injection well.

Introducing Equation (2), expressed in terms of Q_{out} , in Equation (3) and considering the different assumptions presented above, the general equation used for modeling tracer injection is:

$$\pi r_w^2 h_w \frac{\partial C_w}{\partial t} = Q_{in} (C_{in} - C_w) + Q_t^m (C - C_w) \quad (7)$$

Equation (7) shows that the well-aquifer system acts similarly to a dual-porosity system. The injection can thus have some influence on tracer test results due to the capture of tracer in the well bore and gradual release into the aquifer. This may lead to artificially enhanced concentration attenuation and tailing at the observation well [Brouyère, 2001].

4 Numerical model

4.1 Finite-difference approximation

Equation (7) is evaluated numerically, using classical finite-difference approximations:

$$C_w \approx \tilde{C}_w = \omega_w C_w(t + \Delta t) + (1 - \omega_w) C_w(t) \quad (8)$$

$$\frac{\partial C_w(t)}{\partial t} \approx \frac{C_w(t + \Delta t) - C_w(t)}{\Delta t} \quad (9)$$

ω_w is a time weighting factor, an implicit scheme ($\omega_w = 1$) being used to guaranty the stability of the numerical computations, and Δt is the computation time step.

From the groundwater flow simulation performed prior to execution of the transport problem, the variation of water level in the injection well is linearized on the time step Δt , as follows:

$$h_w \approx \tilde{h}_w = \omega_w h_w(t + \Delta t) + (1 - \omega_w) h_w(t) \quad (10)$$

where $h_w(t)$ and $h_w(t + \Delta t)$ are the water levels in the well computed at time t and $t + \Delta t$.

These approximations are introduced in Equation (7), giving the following expression:

$$\left[\frac{\pi r_w^2 \tilde{h}_w}{\Delta t} + \omega_w (Q_{in} + Q_t^{in}) \right] C_w(t + \Delta t) = \left[\frac{\pi r_w^2 \tilde{h}_w}{\Delta t} - (1 - \omega_w) (Q_{in} + Q_t^{in}) \right] C_w(t) + Q_{in} C_{in} + Q_t^{in} C \quad (11)$$

With the initial condition for C_w and the appropriate values for the injection rate and concentration on the computation time step, Equation (11) fully determines the time evolution of tracer concentration in the injection well.

4.2 One-dimensional representation of the injection well

In some cases, well screens extend over several meters, along which the hydraulic conductivity may vary by orders of magnitude, leading to a non-uniform distribution of injected water and tracer fluxes. In the SUFT3D finite element code [Carabin and Dassargues, 1999, Brouyère, 2001], the approach proposed by Sudicky *et al.* [1995], for

modeling wells using one-dimensional finite elements, is used for distributing the injected water “naturally” among the well nodes. From flow computational results on the current time step, if n_w nodes connect the injection well and the aquifer, the different flow rates terms are split into n_w components. For example, the injection rate Q_{in} is split into n_w terms q_{in}^K , with $Q_{in} = \sum_{K=1}^{n_w} q_{in}^K$. Considering these distributed flow rates, Equation (11) can be written:

$$C_w(t + \Delta t) = \frac{1}{R_w} \left(Q_{in} C_{in} + \sum_K q_t^K C_K(t) + B_w C_w(t) \right) \quad (12)$$

$$\text{with } R_w = \frac{\pi r_w^2 \tilde{h}_w}{\Delta t} + \omega_w \sum_K (q_t^K + q_{in}^K) \text{ and } B_w = \frac{\pi r_w^2 \tilde{h}_w}{\Delta t} - (1 - \omega_w) \sum_K (q_t^K + q_{in}^K)$$

$C_K(t)$ is the concentration in the transit flow rate q_t^K at node K connecting the aquifer and the well. It is taken to be in a fully explicit form in order to relax the dependency of computed concentrations in the aquifer at one connecting node on concentrations in the aquifer at other connecting nodes. This does not influence the stability of the numerical scheme, as the explicit evaluation is performed on a source term and not directly in the transport equation. In this case, Darcy fluxes are computed at the nodes located at the well-aquifer interface, based on flow conditions prevailing before injection begins.

Using a general operator $T_{3D}(C)$ to represent the numerical form of the transport equation in the aquifer, the following expression describes the implementation of the injection well in the 3D simulator as a simple source/sink term:

$$T_{3D}(C) - \sum_{K=1}^{n_w} (q_{out}^K C_w - q_t^K C_K(t)) = 0 \quad (13)$$

5 Experimental validation

Typical concentration evolutions monitored in the field during field tracer experiments in alluvial deposits and subsequently modeled with SUFT3D are presented as an illustration of the adequacy and accuracy of the modeling approach with respect to field observations.

The studied site is located in the alluvial aquifer of the river Meuse near Liège in Belgium. A detailed hydrogeological study including groundwater flow and transport modeling was conducted to study the influence on tracer test results of the injection procedure, local flow conditions and tracer characteristics [Brouyère, 2001]. Eight injection wells, located at distances ranging from 4.5 m to 50 m from the pumped well (extraction rate: 52.6 m³/h) were used for the tracer experiments. Injection wells have a radius r_w of 2.5 cm and the average length of the water column h_w in the wells was 7 m ($V_w \approx 13.7$ l). During each injection, a water circulation was performed in order to homogenize the tracer concentration and to obtain samples at the injection point. To illustrate the proposed methodology, results are presented for well Pz5, located 25 m away from the pumping well (for details see Brouyère, 2001). Table 1 summarizes information relative to these injections. Figure 3 shows concentration evolutions monitored in Pz5 together with corresponding profiles computed with SUFT3D. Concentrations are normalized with respect to the concentration in the injected tracer fluid.

During phase I, small volumes and flow rates were used for both the tracer injection and the water flush. During phase II, the flush volume and rate were relatively large. During phase III, it was decided to perform a “long duration injection”, without any flush afterwards. For technical reasons, the injection rate was reduced during the injection. Concentration evolutions were adjusted considering r_w and α_w as fitting parameters. For r_w , a value of 2.5 cm was used, equal to the actual well radius, while for the distortion coefficient α_w , a value of 11.5 was found. This last value is relatively high. However, the distortion coefficient may

be viewed as a “correction” factor used to fit the actual transit flow rate across well screens. Furthermore, all injection wells were drilled with a bit diameter ($d_{drill}=11.5$ cm) larger than the casing ($d_{casing}=5$ cm). The annular space, filled with a gravel pack of high hydraulic conductivity, may induce a strong convergence of flow lines around the well.

For phases I and II, the maxima of concentrations observed at the end of the tracer fluid injection and the decrease of concentration observed during the water flush are well reproduced. During phase II, the tracer back-migration observed in Pz5 is also well reproduced. During phase III, the concentration in Pz5 stabilizes at a value lower than in the injected fluid ($C_w^* < 1$), indicating that a transit flow rate exists and contributes to flush the tracer. When the injection rate is reduced, the transit flow rate is increased, contributing to enhanced dilution of the tracer in the injection well and the stabilization of concentration at a lower level. In addition, the computed concentration evolution remarkably reproduces the dynamic variation of concentration with respect to the equilibrium between injection and transit flow rates, suggesting that Equation (4), linking the injection and transit flow rates, is accurate.

6 Conclusions

A new conceptual and mathematical approach is proposed for modeling tracer concentration evolution in wells and the solute mass flux leaving or crossing the well at the screen level. It is validated by modeling concentration evolutions monitored in the field during tracer experiments. Contrary to previous approaches, this physically-based model considers all processes that can have some influence on solute exchange between a well and the surrounding aquifer: finite volumes of tracer and flush, mixing and skin effects, back-diffusion of the tracer in the well bore, heterogeneity of aquifer materials close to the injection well.

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Figure captions

Figure 1. Schematic representation of the well-aquifer system and exchanged fluxes

Figure 2. Modification of groundwater flow lines in the vicinity of the well,
according to the injection flow rate Q_{in}

Figure 3. Comparison between concentration evolutions monitored in Pz5 and modeled with
the SUFT3D code

Table 1. Description of the injections performed in Pz5

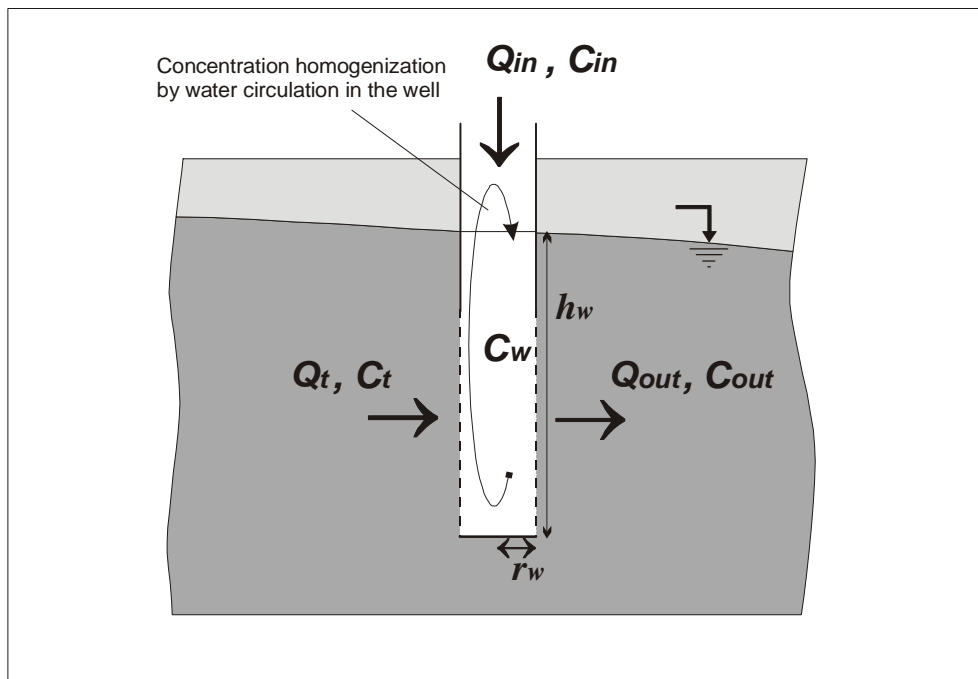


Figure 1

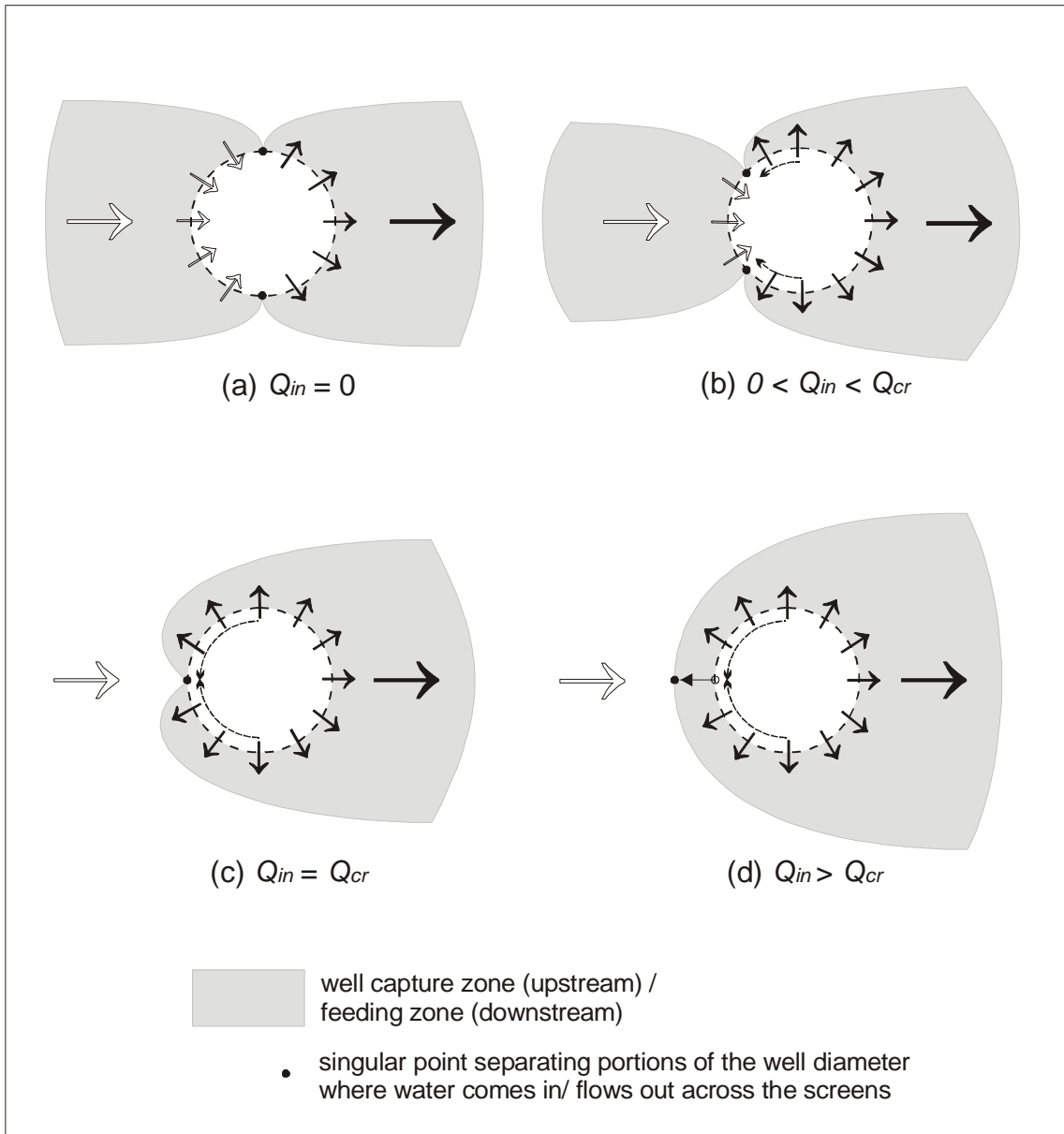


Figure 2

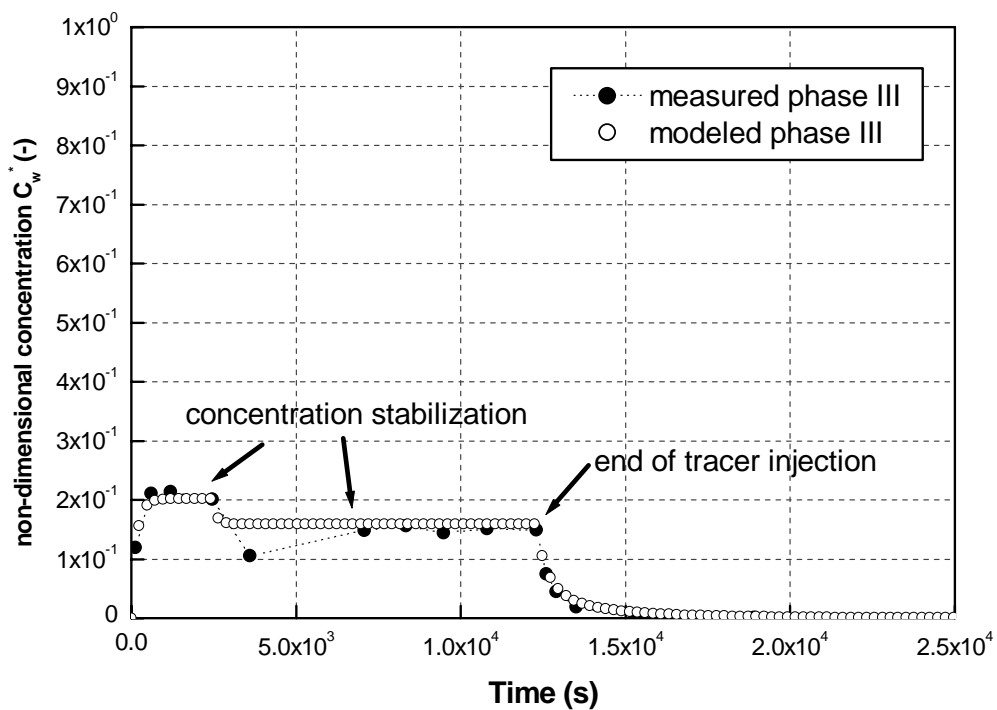
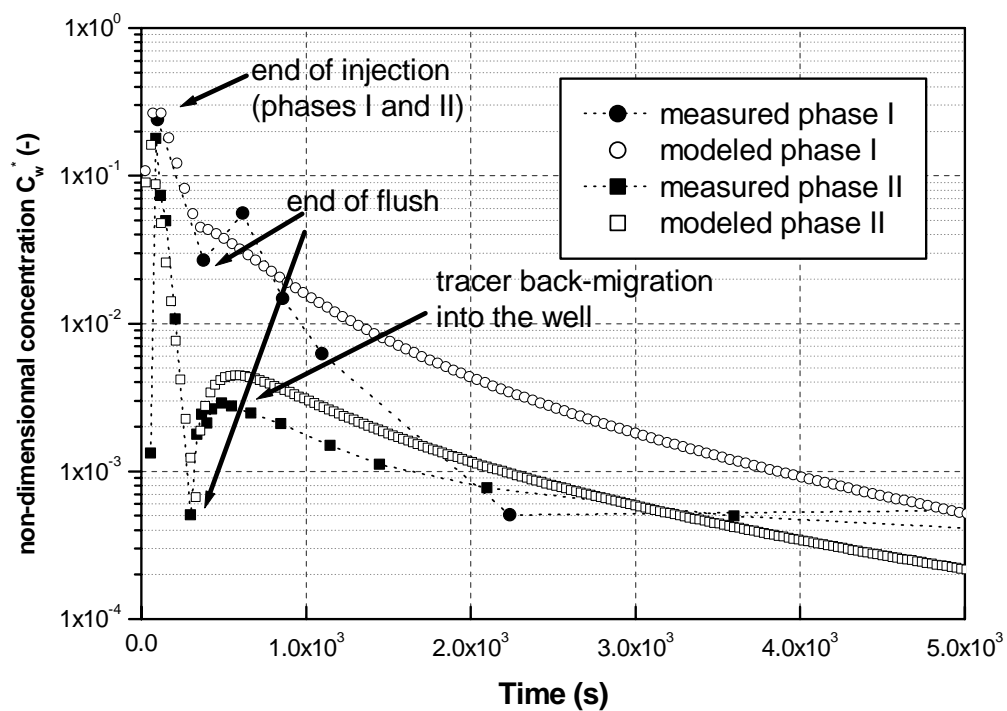


Figure 3

Tracer	Phase	V_{inj} (m³)	T_{inj} (s)	Q_{inj} (m³/s)	V_{fl} (m³)	T_{fl} (s)	Q_{fl} (m³/s)
Naphthionate	I	0.007	100	7.0×10^{-5}	0.014	240	5.75×10^{-5}
	II	0.003	52	5.77×10^{-5}	0.100	268	3.73×10^{-4}
	III	0.048 + 0.152	2400 + 9900	2.0×10^{-5} + 1.54×10^{-5}	--	--	--

Table 1