Numerical modelling of hydromechanical coupling :

# permeability dependence on strain path

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# 2 hydromechanical coupling applications

- Permeability in EDZ (Benoit Pardoen thesis)
- Permeability in compacted bentonite (Anne-Catherine Dieudonné thesis)
- Simple hydromechanical coupling, however highly nonlinear !
- Numerical modelling with the finite element code LAGAMINE, developed at University of Liege : multiphysical coupling and failure
- Experiments developed by ANDRA at Bure LSMHM URL

#### **Excavation Damaged Zone (EDZ)**





Fracturing & permeability increase (several orders of magnitude)

Opalinus clay in Switzerland (Bossart et al., 2002)



- Fracturing



#### **Finite element methods**

#### - Classical FE

Mesh dependency

Need to introduce an internal length scale for a correct modelling of the post-peak behaviour.



- Regularisation

Macro Ω:

Enrichment of the kinematics :



#### Large-scale experiment of gallery ventilation (SDZ)

Characterise the effect of gallery ventilation on the hydraulic transfer around it.

ightarrow drainage / desaturation

 $\rightarrow$  exchange at gallery wall







#### **Evolution of intrinsic water permeability**

Various approaches: deformation, damage, cracks...

- Relation to deformation

Volumetric effects = increase of porous space (Kozeny-Carman)

$$k_{w} = k_{w,0} \frac{\left(1 - \phi_{0}\right)^{\xi_{1}}}{\phi_{0}^{\xi_{2}}} \frac{\phi^{\xi_{2}}}{\left(1 - \phi\right)^{\xi_{1}}} \qquad \qquad \mathcal{E}_{v} = \frac{\mathcal{E}_{i}}{3}$$

- Fracture permeability

Cubic law for parallel-plate approach (Witherspoon 1980; Snow 1969, Olivella and Alonso 2008)







Localised deformation Fracture initiation

- <u>Empirical law</u> (Pardoen et al., under review)

Related to strain localisation effect Permeability variation threshold

$$k_{w,ij} = k_{w,ij,0} \left( 1 + \beta_{per} \left\langle YI - YI^{thr} \right\rangle \hat{\varepsilon}_{eq}^{3} \right) \qquad \qquad YI = \frac{II_{\hat{\sigma}}}{II_{\hat{\sigma}}^{II}}$$

Extension



#### Hydraulic boundary condition for exchanges at gallery wall

- <u>Classical approach</u> Instantaneous equilibrium (Kelvin's law)

$$RH = \frac{\rho_{v}}{\rho_{v,0}} = exp\left(\frac{-p_{c}M_{v}}{\rho_{w}RT}\right)$$

- Experimental

Drainage / desaturation  $\rightarrow$  Progressive hydraulic transfer & equilibrium

- <u>Non-classical mixed boundary condition</u> (Gerard et al. 2008) Liquid water + water vapour  $\overline{q}_w = \overline{S} + \overline{E}$ 

- Seepage flow :  $\begin{cases} \overline{S} = K^{pen} (p_w^{\Gamma} - p_{atm})^2 & \text{if } p_w^{\Gamma} \ge p_w^{air} \text{ and } p_w^{\Gamma} \ge p_{atm} \\ \overline{S} = 0 & otherwise \end{cases}$ 

- Evaporation flow : (Nasrallah and Perre, 1988)

$$\overline{E} = \alpha_v \left( \rho_v^{\Gamma} - \rho_v^{air} \right)$$





#### Modelling of excavation and SDZ experiment



#### **Desaturation EDZ / w reproduction**





The processes taking place under repository conditions include

- Development of swelling strain / pressure
- Evolution of the water retention properties, the permeability...
- Structure changes



→ Complex and strongly coupled <u>multiphysical</u> & <u>multiscale</u> processes !

### PGZ2 in situ experiments

- <u>Objective</u>: characterization of the water saturation process of bentonite buffers <u>under natural conditions</u>.
- PGZ1013: compacted MX-80 bentonite / sand mixture (70/30 in dry mass).
  - ρd0 = 2.06 Mg/m<sup>3</sup> (n = 0.25), ~13% technological void.





Experimental characterization performed in:

- CEA Saclay, France (Gatabin et al. 2016)
- Ecole des Ponts ParisTech, France (Wang 2012, Saba 2013)

Water retention behaviour

## Constitutive model

(Dieudonne et al., submitted)

$$e_w = S_r \cdot e = e_{wm} + e_{wM}$$



 $\blacktriangleright \underline{\text{Macrostructural water ratio}}: e_{wM}(s, e, e_m) = (e - e_m) \left[1 + \left(\frac{s}{a}\right)^n\right]^{-m}$ 

Coupled modelling (HM): 
$$K_W = K_0 \frac{(1-\phi_{M0})^M}{(\phi_{M0})^N} \frac{(\phi_M)^N}{(1-\phi_M)^M}$$
 m<sup>2</sup>

HM formulation Bentonite buffer



HM formulation

# Technological void / interface

- Zero-thickness interface finite element. (Cerfontaine et al. 2015)
- HM coupled formulation for partially saturated interfaces:
  - Absence of contact / contact (penalty method).
  - Transversal fluxes computed according to the pressure drop between both interface sides and the inside.



# Numerical results

- Evolution of the bentonite buffer:
  - Very high transmissivity if contact, lower if technological void.
  - Preferential hydration from the bottom in the early process.



Borehole diameter = 101.3 mm Initial buffer diameter = 94 mm Initial technological void = 13%

## Numerical results



Suction, s: MPa

# Numerical results

- Strong influence of the water retention model:
  - Higher degree of saturation if a constant WRC is used.
  - Saturation kinetics overestimated if a constant WRC is used.



Results after 1 weeks

# Conclusion

- The challenges : highly non linear coupling terms
- Permeability evolution based on micro scale considerations
- EDZ : fractures in an anisotropic context Permeability evolution with fracturing
- Bentonite : free swelling vs confined swelling, permeability evolution
- Adoption of a double-structure water retention curve to model the evolving properties of the bentonite buffer.
- Use of interface finite elements to model the progressive closing of technological voids.

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