

***The development of Structural Fire Engineering
over the past 25 years
and issues for the future***

Jean-Marc Franssen

***The development of Structural Fire Engineering
over the past 25 years !
and issues for the future***

=> Back to 1990....

Or 1982?

Or earlier?

EUROPEAN CONVENTION FOR CONSTRUCTIONAL STEELWORK
INTERNATIONAL ASSOCIATION
FOR BRIDGE AND STRUCTURAL ENGINEERING

STABILITY OF STEEL STRUCTURES

PRELIMINARY REPORT

LIEGE

13 - 14 - 15
APRIL 1977

Second International Colloquium

IN CO-OPERATION WITH
STRUCTURAL STABILITY RESEARCH COUNCIL
COLUMN RESEARCH COMMITTEE OF JAPAN

Critical appraisal of the plastic design rules of ECCS on the basis of the numerical investigation of a braced 25-storey building, by Ch. MASSONNET, A. ANSLIJN, A. RIGON - Liège, Belgium	529
Stability of structural members in the french specifications for plastic design of steel structures, by Y. LESCOUARC'H - France	535
Stability of haunched rafters, by L.J. MORRIS, J.A. PACKER - England . . .	539
Ultimate load carrying capacity of steel arches with initial imperfections, by T. SAKIMOTO, S. KOMATSU - Japan	545
In-plane strength of arch bridges subjected to vertical and lateral loads, by S. KURANISHI, T. YABUKI - Japan	551
Inelastic snap-through buckling of steel reticular tied arches, by A. FONTANA, F. SCIROCCO - Italy	557

THEME 10.- Shells 563

An engineering approach to the problems of plastic and elastic shells under axial pressure, by M.S. EL NASCHIE - Saudi Arabia	565
Buckling of circular cylindrical shells under combined axial compression and internal pressure, by H. SAAL - Germany	573
Buckling of axially compressed thin-walled cylindrical shells with asymmetric imperfections, by Bo L.O. EDLUND - Sweden	579
Some complements to the ECCS design code concerning isotropic cylinders, by M. ESSLINGER, B. GEIER - Germany, J.G.M. WOOD - England	589
Model investigation of the collapse of a steel water tower, by D. VANDEPITTE - Gent, Belgium	599
Use of computer programs Bosor 4 and 5 in the stability analysis of two civil engineering steel shell structures, by Ch. MASSONNET, R. BALTUS - Liège, Belgium	609
Some experimental results on the elastic-plastic buckling of thin torispherical and ellipsoidal shells subjected to internal pressure, by G.D. GALLETLY - England	619
The stability of buried shells under surface loading, by P.S. BULSON - England	627

THEME 11.- Special problems 633

Non linear analysis of metal structures, by F. FREY - Liège, Belgium	635
Dynamical plate buckling with structural damping, by P. SCHROEDER - Luxembourg	641

The stability of braced and unbraced frames at elevated temperatures,
by J. WITTEVEEN, L. TWILT, F.S.K. BIJLAARD - The Netherlands 647

APPENDIX TO THEME 5.- 657

Design of cold formed steel stiffened elements, by A. HASEGAWA, Japan
N.C. LIND - Canada 659

1) Behaviour of materials

A lot has been done already, at least on « traditional » materials

A lot has been lost (or ignored)

Sonderforschungsbereich 148

1) Behaviour of materials

A lot is still being done.

Please use published recommendations.

RILEM recommendations

1) Behaviour of materials

Old fashioned approach

- *Take material model at room temperature*
- *List the parameters of the model*
- *Measure these parameters at elevated temperature*

Better approach

- *Choose a material model at elevated temperature*
- *List the parameters of the model*
- *Measure these parameters at elevated temperature*

Material behaviour has been « normalised » (in Eurocodes)

Is it a good thing?

Workshop on material properties at elevated temperatures
ECCS, Arnhem, The Netherland, June 12, 1986

2) Tests on structural members or structures

Tests on small scale structures?

Not for all materials (OK for metals)

Not so popular anymore

2) Tests on structural members or structures

Test on large structures (Cardington)

Very expensive

What to look for?

=> Not so common

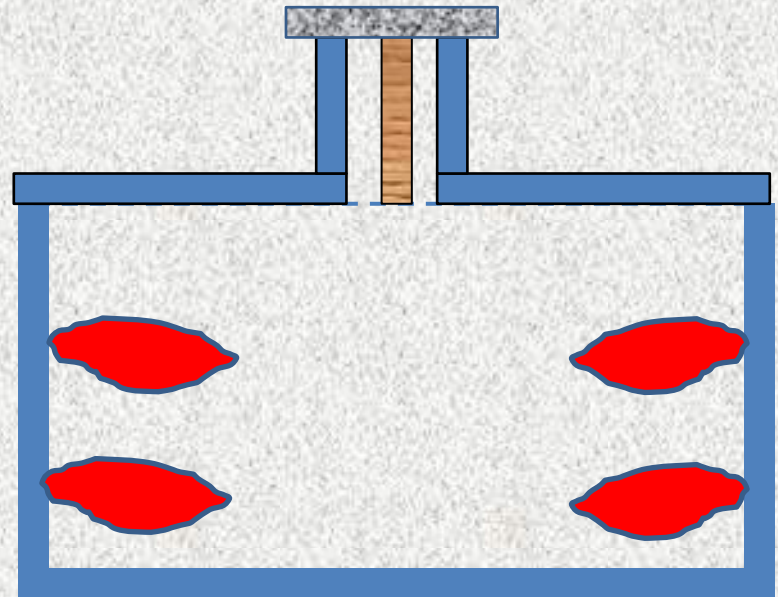


2) Tests on structural members or structures

Test on elements?

- Q1: Do we need it or not?
- Q2: ISO fire or not?
- Use of plate thermometer
- ISO 17025?

“General requirements for the competence of testing ... laboratories”



Make your tests in a laboratory that has accreditation ISO 17025

3) Tabulated data

- Have been there for a while
- No significant breakthrough

Résistance au feu R_i	Dimensions minimales (mm)			
	Largeur des poteaux b_{min} / distance axe-parement a des barres principales			
	Poteau exposé sur plus d'un côté			Poteau exposé sur un seul côté
		$\mu_{s1} = 0.6$	$\mu_{s1} = 0.7$	$\mu_{s1} = 0.7$
1	2	3	4	5
R 30			200/32 300/27	155/25
R 60			250/48 350/40	155/25
R 90	200/31 300/25	300/45 400/38	350/53 450/40**	155/25
R 120	250/40 350/35	350/45** 450/40**	350/57** 450/51**	175/35
R 180	350/45**	350/63**	450/70**	230/55
R 240	350/61**	450/75**	-	295/70

**

Minimum 8 barres

Pour les poteaux en béton précontraint, il convient de noter l'augmentation de la distance de l'axe au parement selon 4.2.2. (4).

4) Simple calculation methods

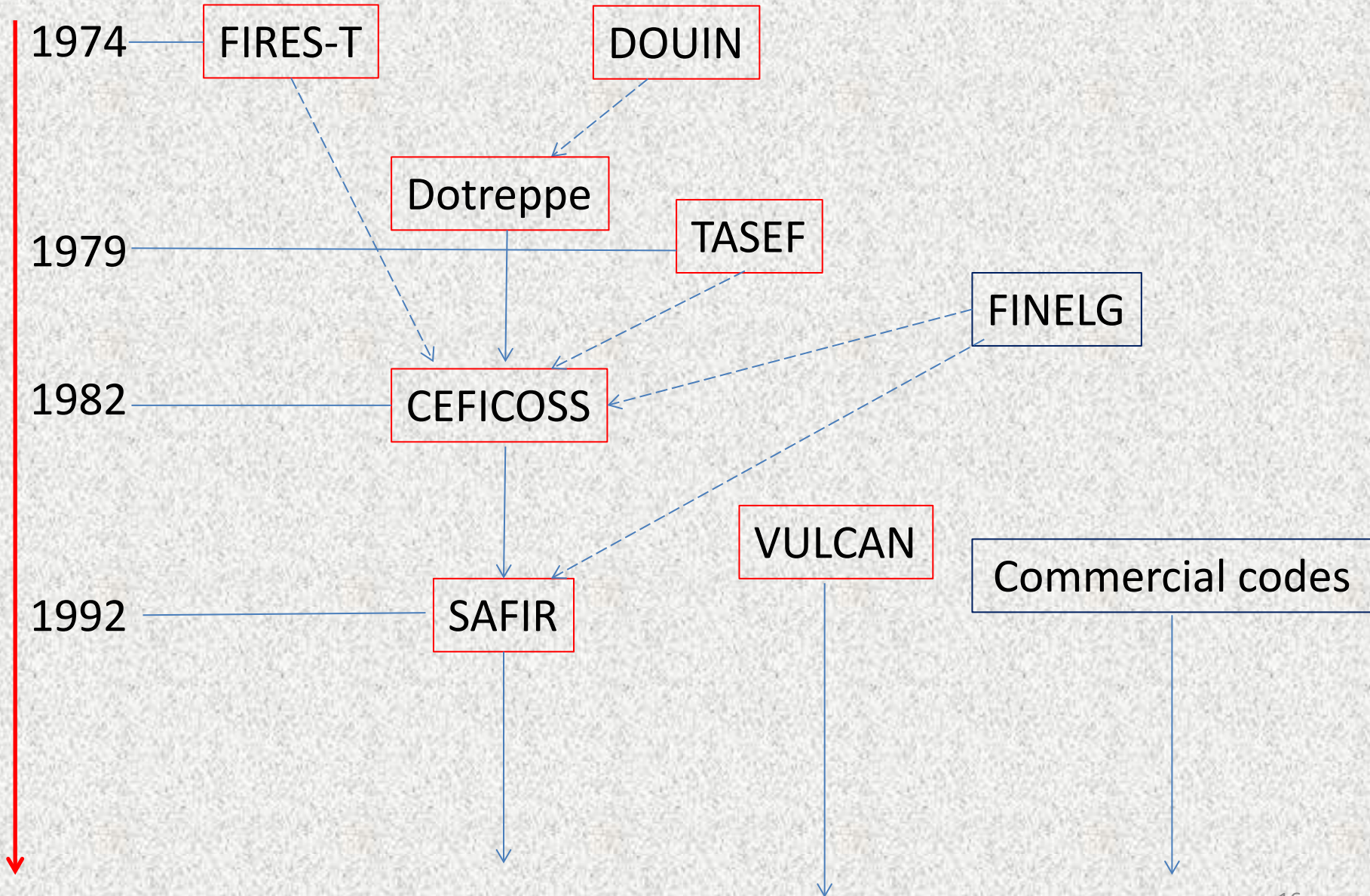
- Have been there for a while.

The little red book?

European Recommendation for the Fire Safety of Steel Structures,
ECCS, 1983.

- No significant breakthrough

5) Advanced calculation models



Aims and capabilities of numerical modelling

- To reproduce a standard fire test (beam, then column)
- To analyse 2D frames (ISO curves, then other *increasing* curves)
- To represent 3D frames
- To combine different finite element types (beams, shells)
- Dynamic analyses
- Analyse local details (joints – volumic elements)

Failure mode \leq Fire resistance time

Q1: Which materials can we use in simulations?

A priori all of them

BUT

on the condition that we know the properties of the model.

⇒ Know your model and its limits

Challenges for thermal calculation

- Contact resistance between two materials.
- Effects of large displacements (the structure moves to the fire).
- Moisture
- Behaviour during cooling
- Changes of geometry (charring, expansion, spalling)



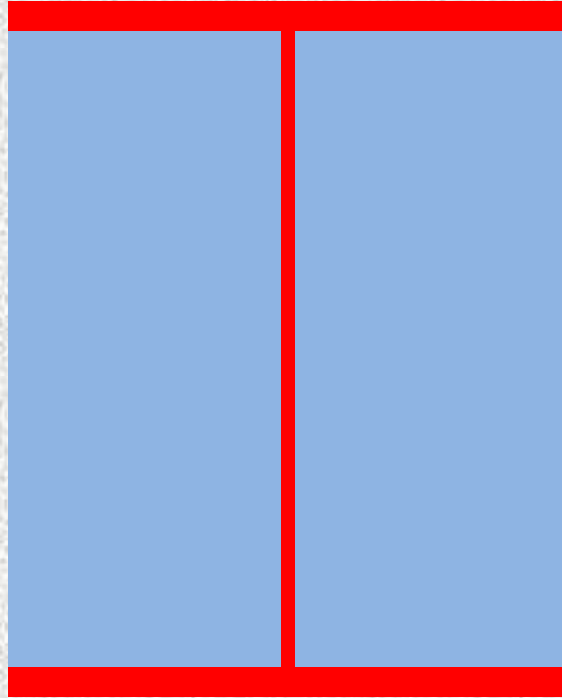
Q2: What kind of structure can we model?

Practically none

except if....

we tested a similar one before

One example: composite steel-concrete column.



The solution is easy:

Just model every possible physical phenomena.

- ✓ 3D solid elements,
- ✓ changes of geometry,
- ✓ contacts,
- ✓ full thermo – hydro – visco – dynamic - mechanical coupling.
- ✓ ...



Good luck!

Have we made some progress in structural fire modelling?

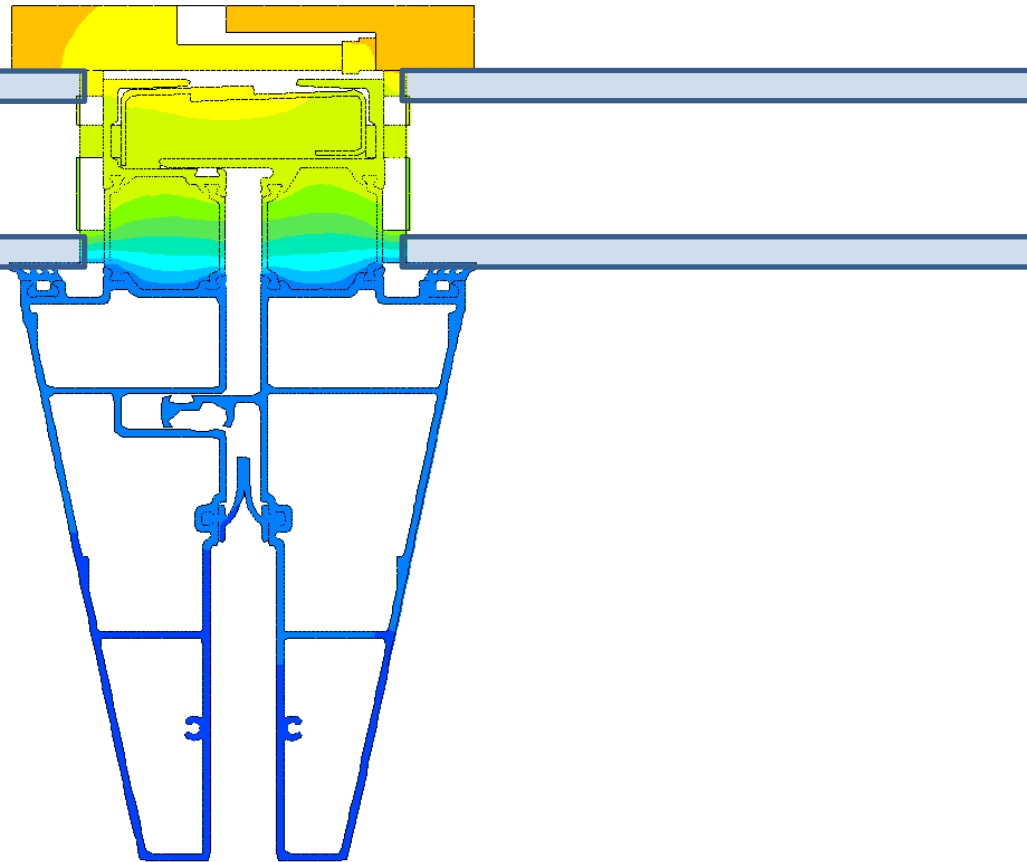
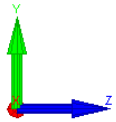
- ✓ Capabilities of the software
- ✓ What is the direction in the stress-strain plane for the next time step, loading or unloading?
- ✓ How are the residual stresses in steel sections influenced by a fire?

Some nice examples (made with SAFIR)

FILE: final
NODES: 5519
ELEMENTS: 8566

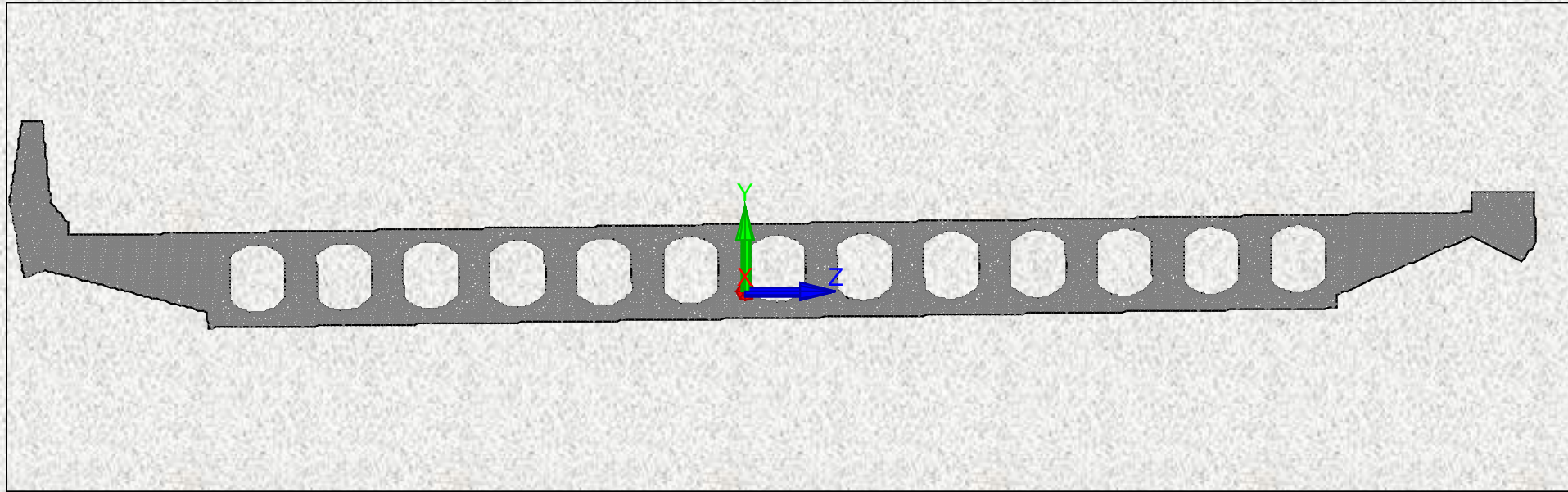
CONTOUR PLOT
TEMPERATURE PLOT

TIME: 3600 sec
>Tmax
1200.00
1100.00
1000.00
900.00
800.00
700.00
600.00
500.00
400.00
300.00
200.00
100.00
0.00
<Tmin



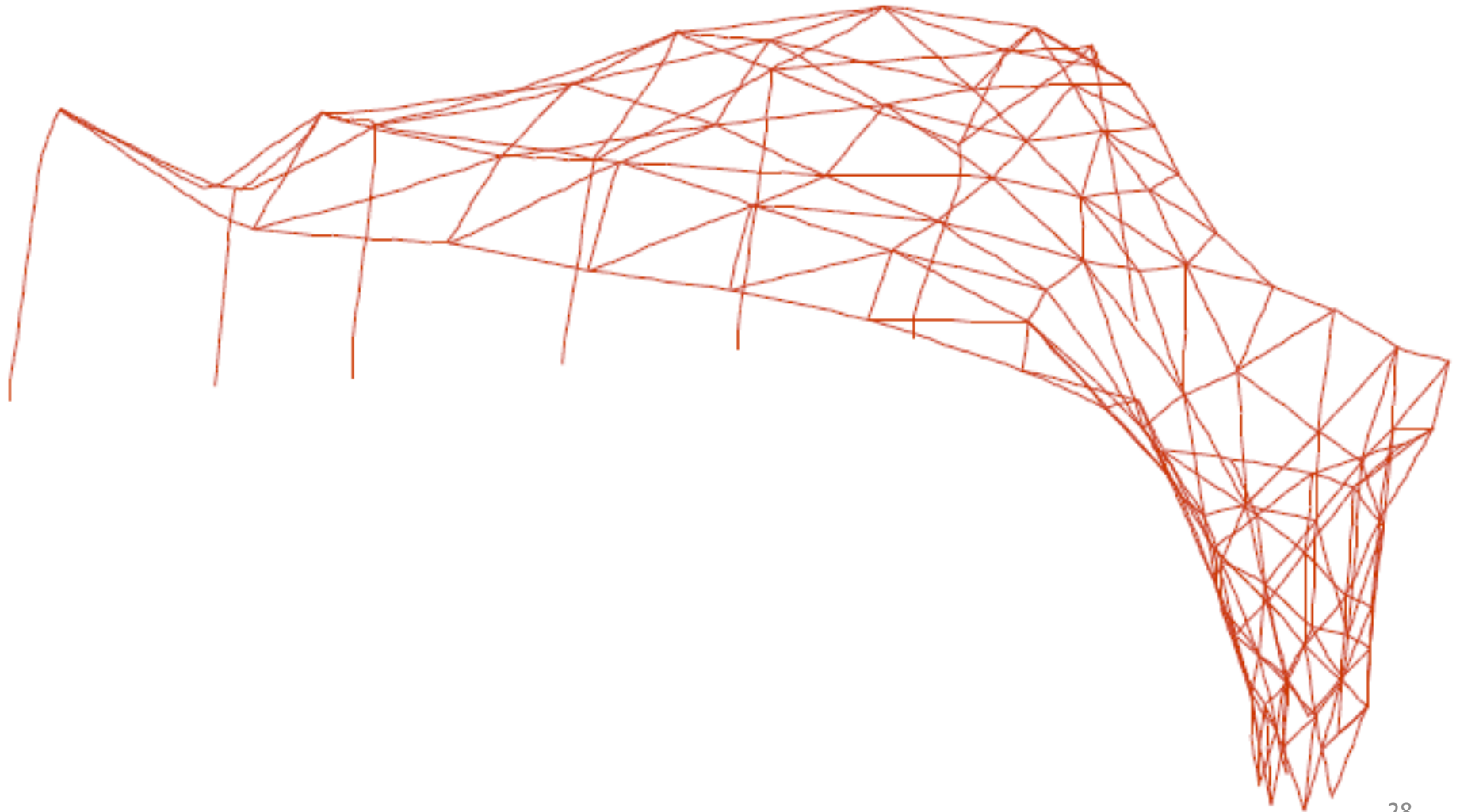
Window frame (courtesy: Permasteelisa)

The deck of a concrete bridge (author unknown)



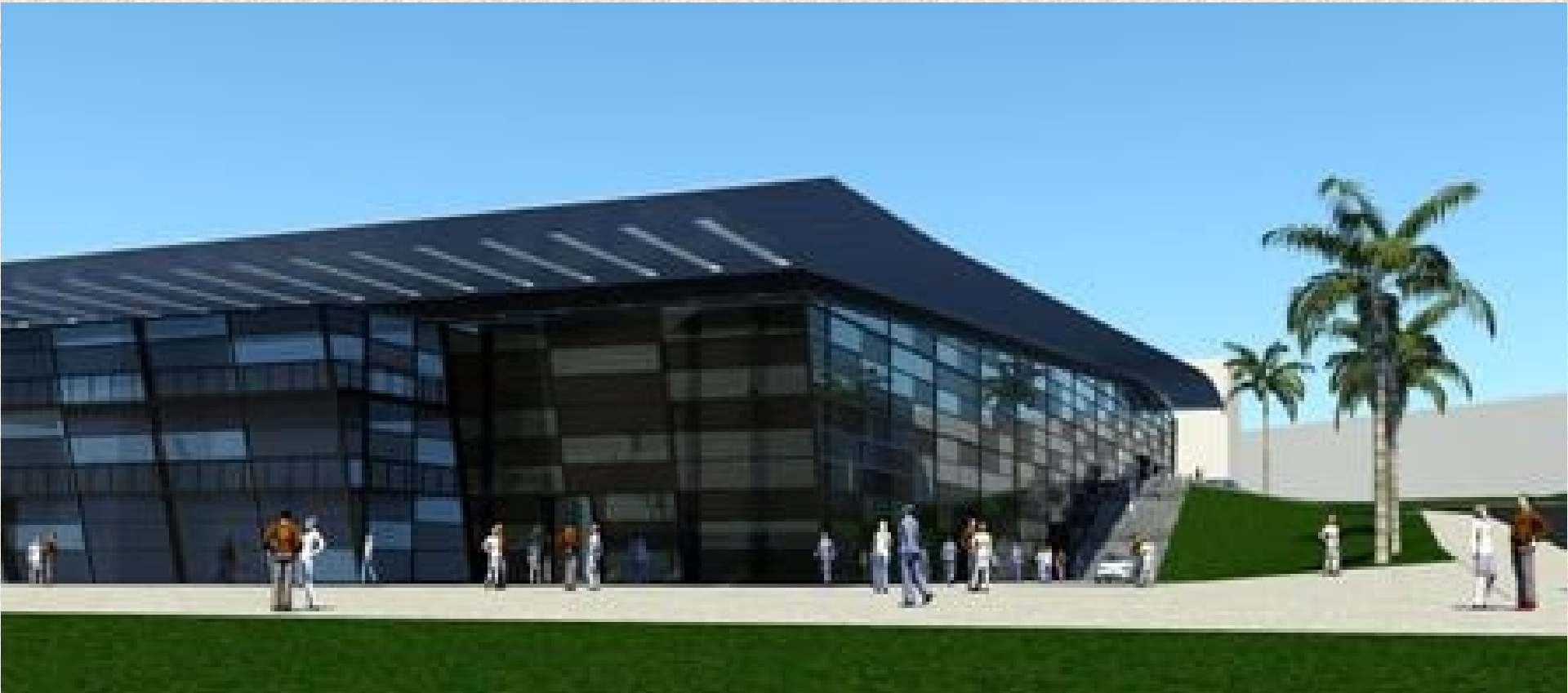
3D eye catcher, Brussels airport

Model: StuBeCo (courtesy Tom Molken)

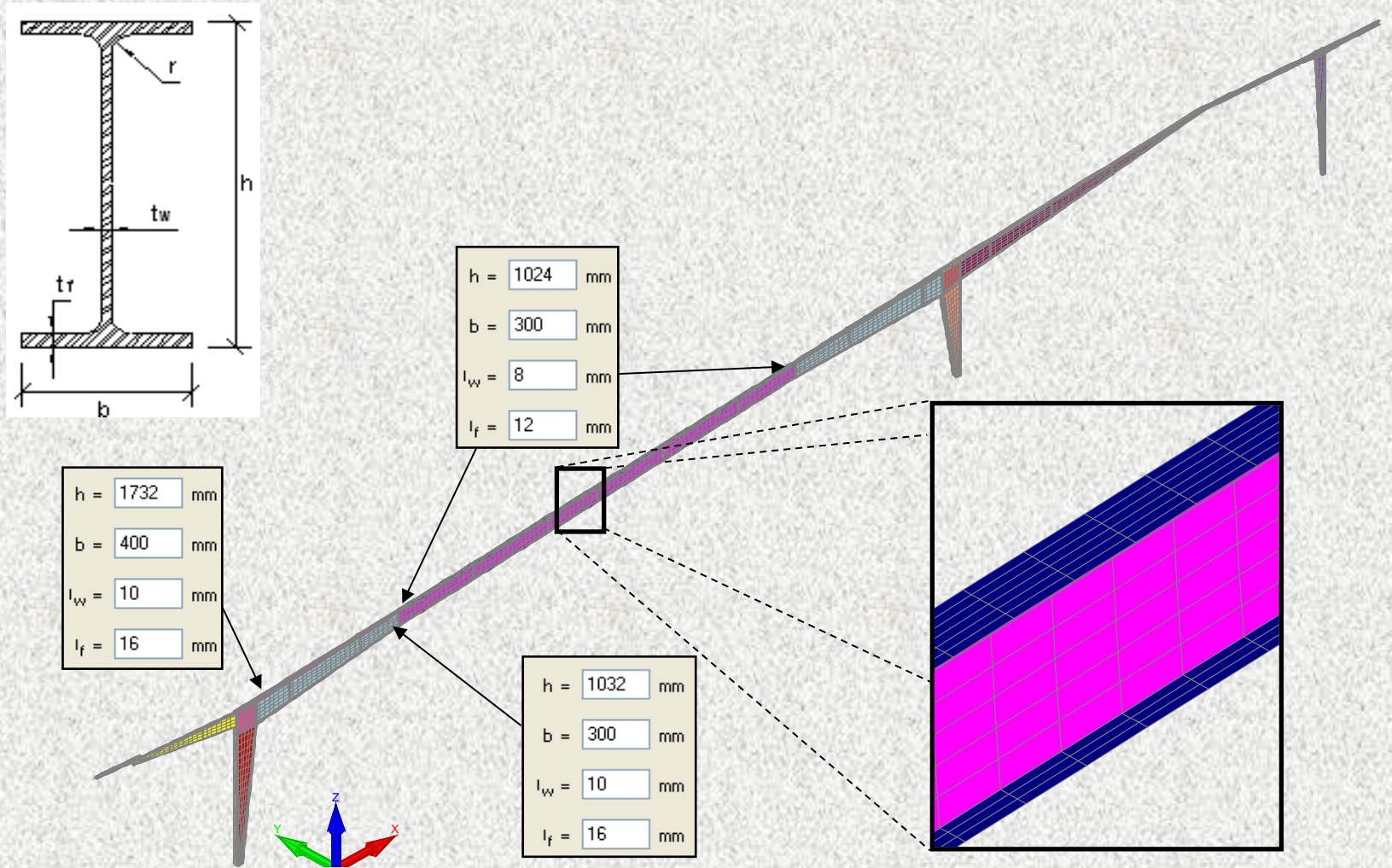


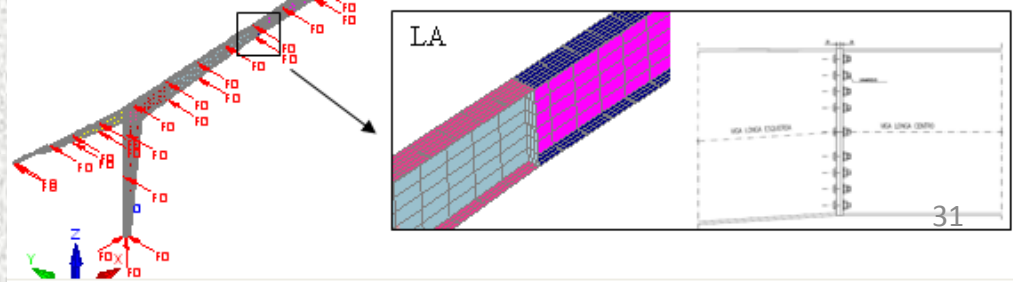
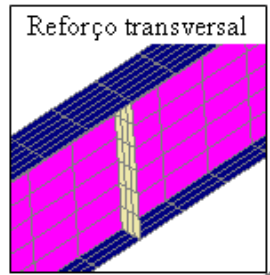
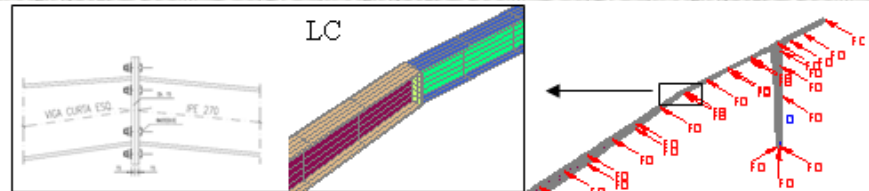
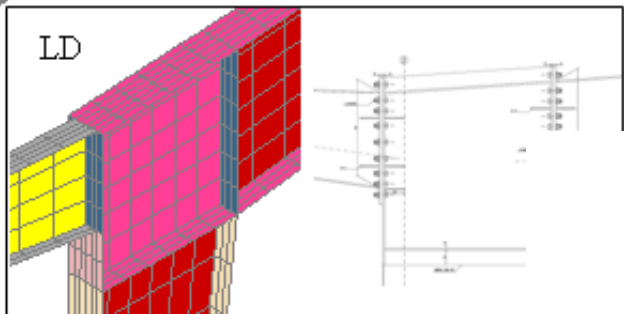
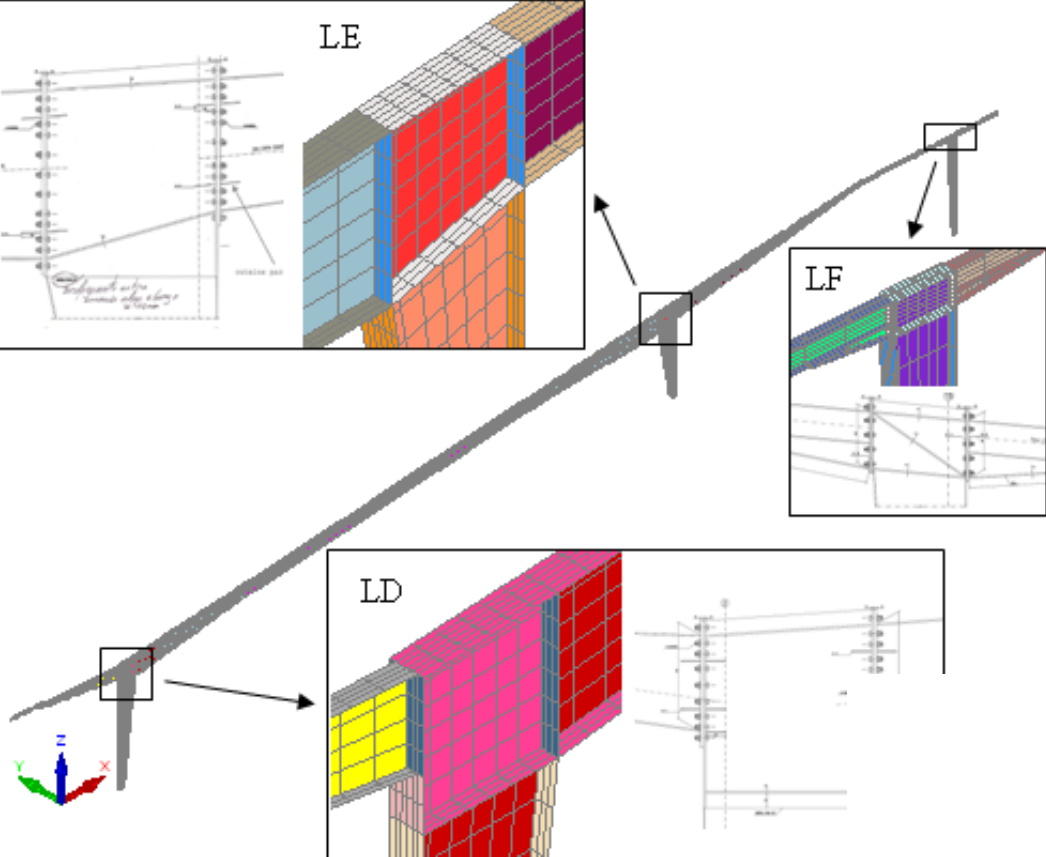
Oeiras Valley Convention Center, Oeiras PT

Luis Neto, arch.



Model: Univ. of Aveiro (courtesy Paolo Vila Real & Nuno Lopes)

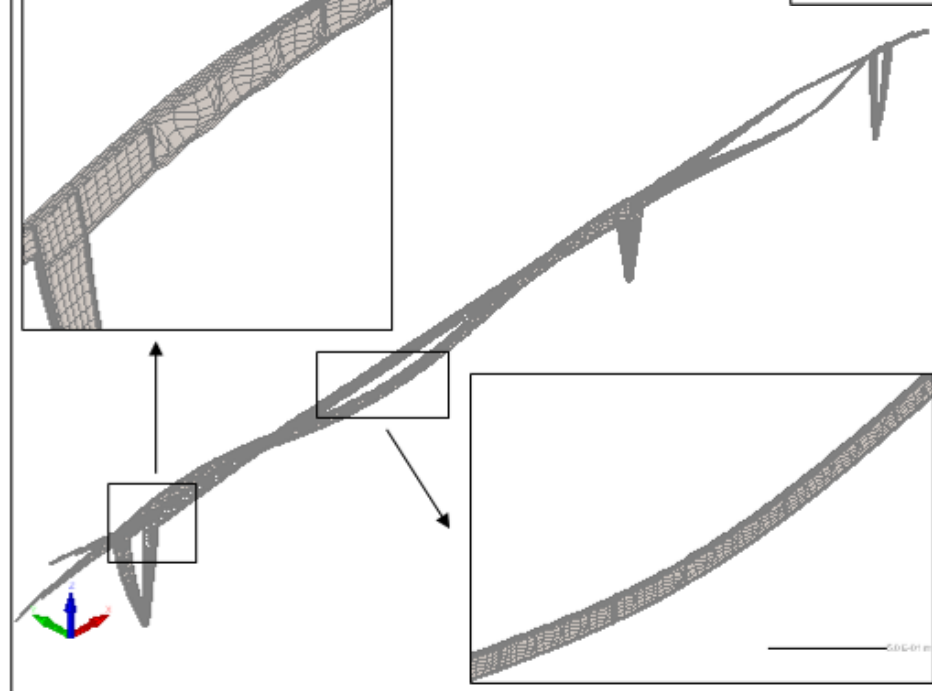
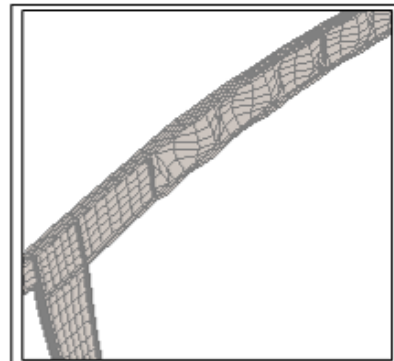
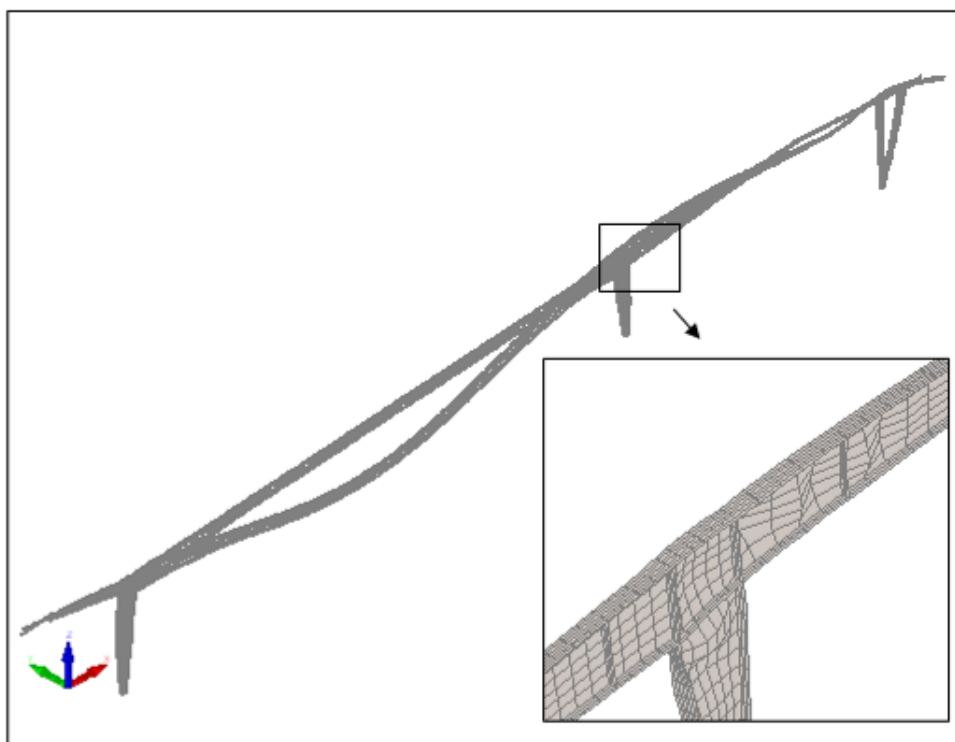




FILE: 0
NO. OF: 049
BEAMS: 0
TRUSSES: 0
SHELLS: 000
SOLES: 0

SHELLS PLOT
DISPLACEMENT PLOT (x 20)

TIME: 2220.14 sec.
Shell Elements



FILE: 0
NO. OF: 049
BEAMS: 0
TRUSSES: 0
SHELLS: 000
SOLES: 0

SHELLS PLOT
DISPLACEMENT PLOT (x 20)

TIME: 2220.51 sec.
Shell Elements

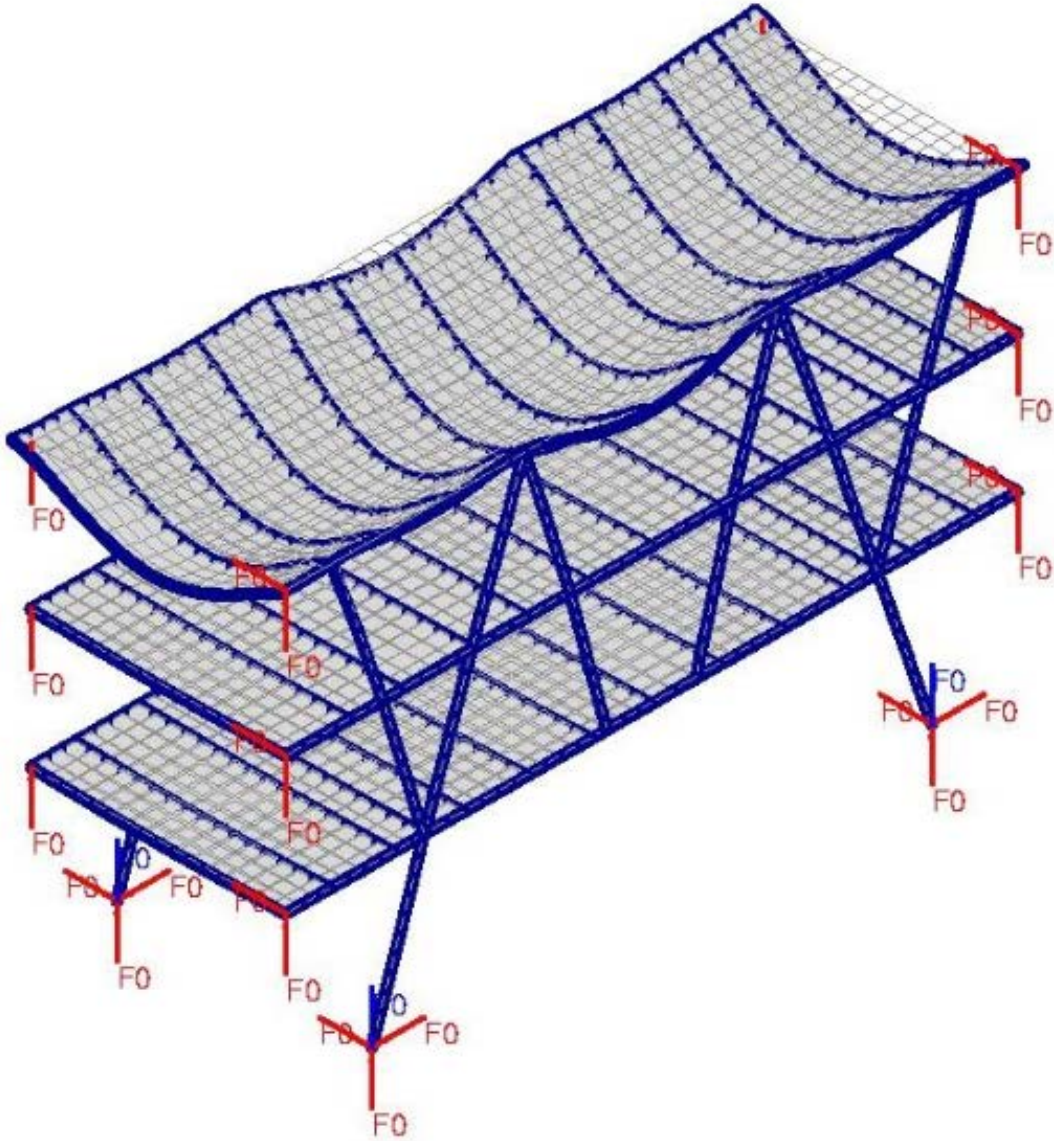




Loterie romande, Lausanne CH
CHE architecture et Design Arch.



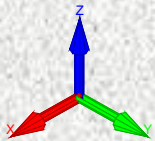
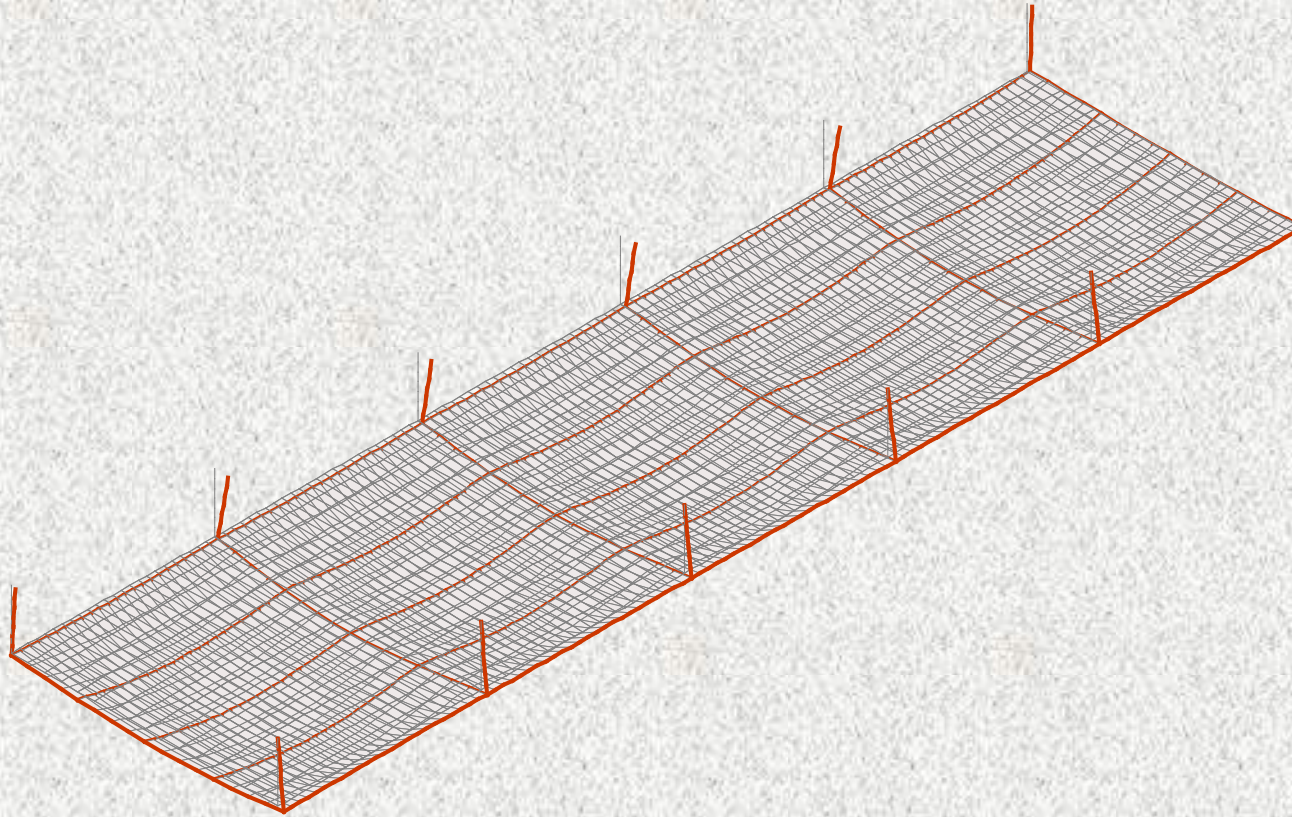
Model: Daniel Willi SA – Montreux CH (courtesy Olivier Burnier)



Japan Tobacco Intl, Geneva CH
SOM & Burckhardt Partner, Arch.



Model: Ingeni (courtesy Lorenzo Lelli)



5.0E+00 m

Diamond 2011.a.1 for SAFIR

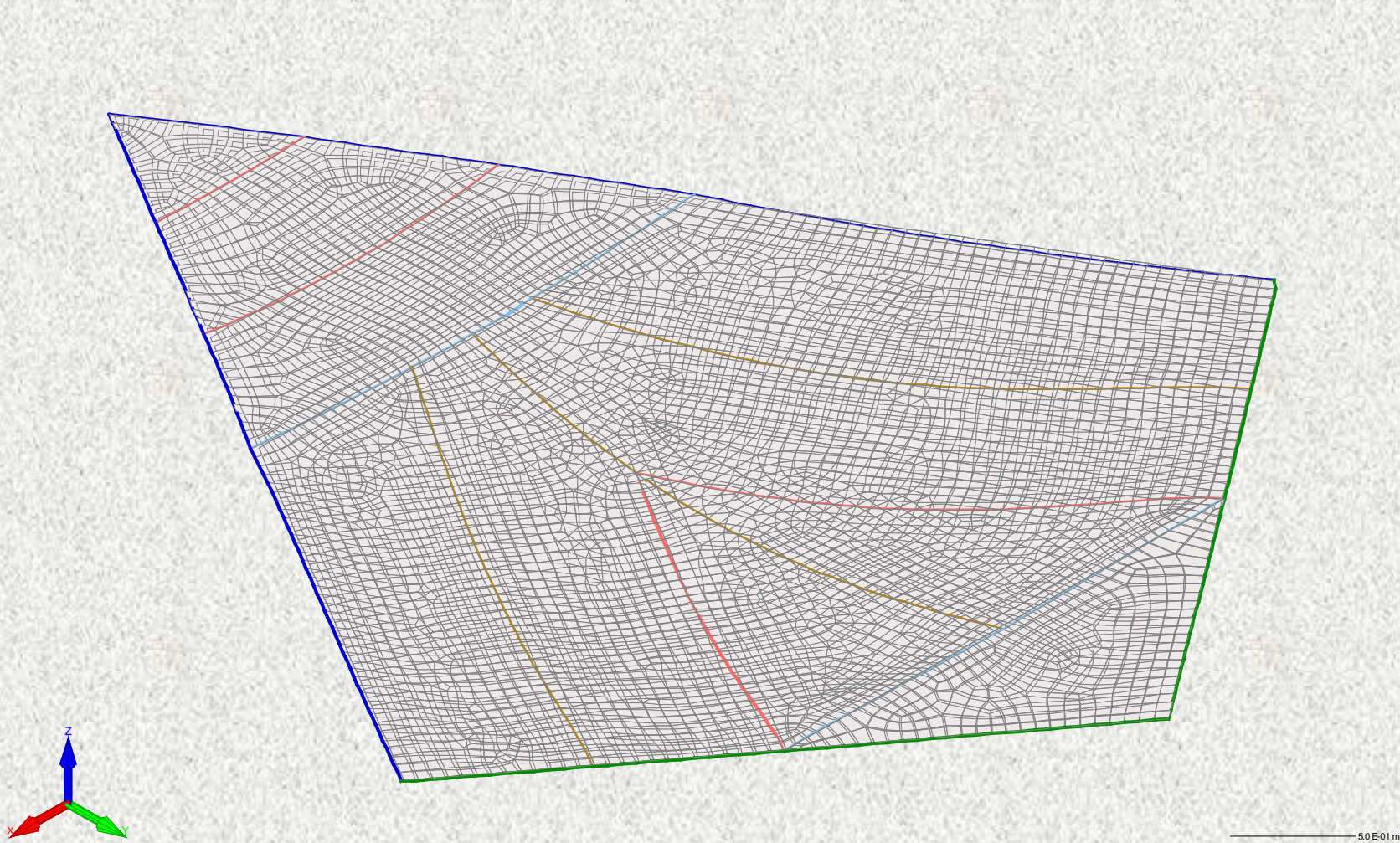
FILE: trave3lel
NODES: 2367
BEAMS: 780
TRUSSES: 0
SHELLS: 1020
SOLLS: 0

BEAMS PLOT
SHELLS PLOT
DISPLACEMENT PLOT (x5)

TIME: 4558.316 sec

Beam Element
Shell Element

Model: Ingeni (courtesy Lorenzo Lelli)



Diamond 2011.a.1 for SAFIR

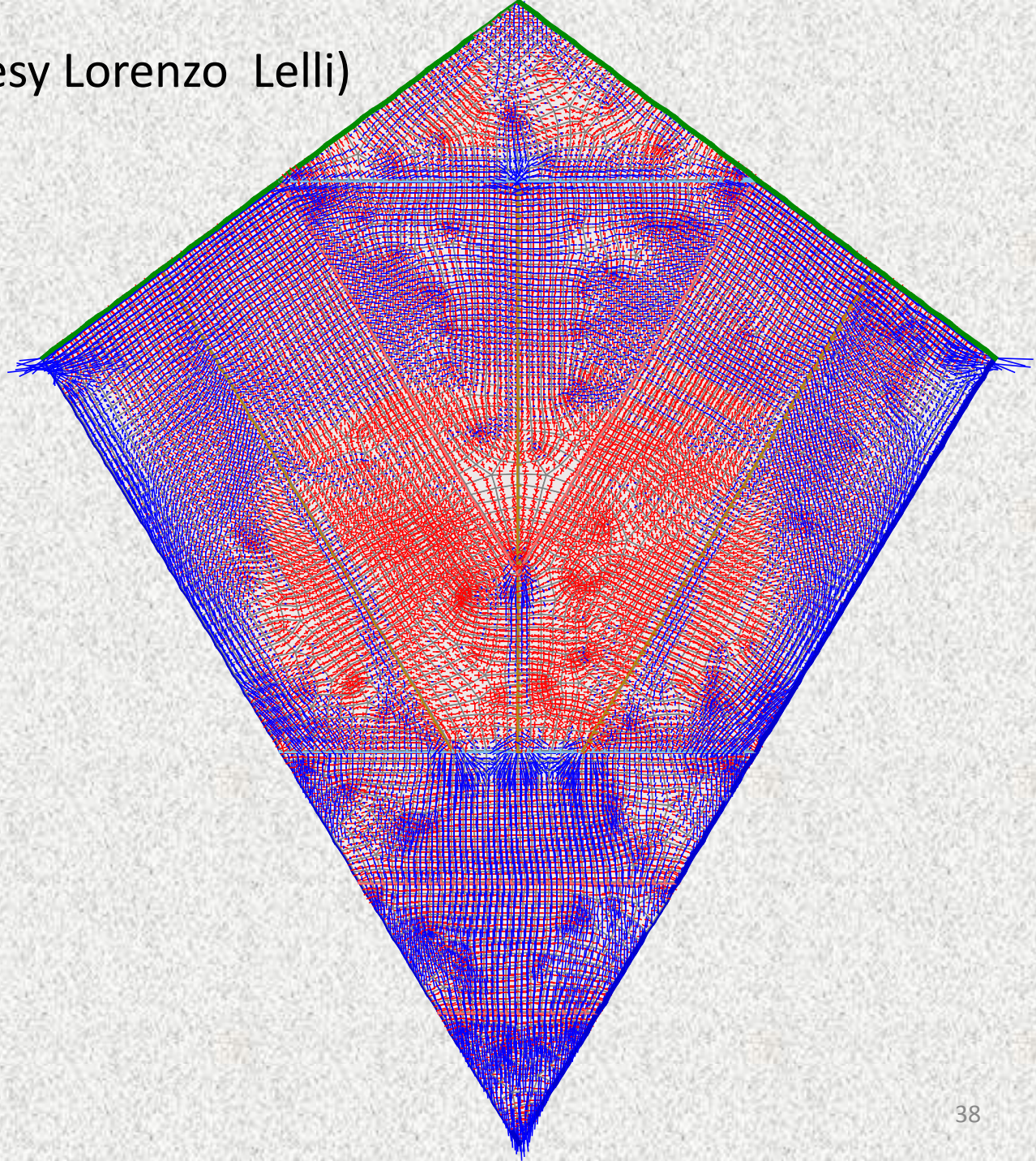
FILE: print6BU
NODES: 5038
BEAMS: 619
TRUSSES: 0
SHELLS: 4218
SOLS: 0

BEAMS PLOT
SHELLS PLOT
DISPLACEMENT PLOT (x5)

TIME: 4542.061 sec

HEAA60COLD.tem
BU900-4COLD.tem
HEAA60HOT.tem
IPEA450HOT.tem
BU900-4HOT.tem
E115HOT.tsh

Model: Ingeni (courtesy Lorenzo Lelli)



Misuse of numerical modelling

Results of simulations are sometimes presented which show extremely ductile behaviour, typically for steel structures.

If several hypotheses which are at the base of the numerical model have been violated, such as Bernoulli hypothesis, small deformation, limited rotations, infinite strength of joints, interpenetration of adjacent elements, descending branch in the stress-strain diagrams, etc, this is in our view a misuse of numerical modelling.

And the future?

I don't know.

Probably:

- Simulation during the cooling phase

- New materials and construction systems.

- Probabilistic aspects

- CFD-FE interaction

- Local fire models

I have some doubts

- Solid mechanics (3D finite elements)

- Prediction of spalling

*9th intl Conf. Structures in Fire
8-10 June 2016, Princeton*

Extended abstracts before December 14, 2015



<https://sif2016.princeton.edu/>