

# Importance of structural damping in the dynamic analysis of compliant deployable structures

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65th International Astronautical Congress  
Toronto, 30 September 2014



# OUTLINE

## INTRODUCTION

- Tape springs

- Types of damping

## OBJECTIVES

## ONE DEGREE OF FREEDOM SYSTEM

## TAPE SPRING - DYNAMIC ANALYSIS

- Without structural damping

- With structural damping

- Comparison

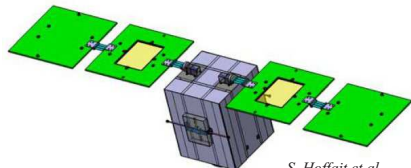
## CONCLUSIONS

# INTRODUCTION - TAPE SPRINGS

**Definition:** Thin strip curved along its width used as a compliant mechanism

## General characteristics:

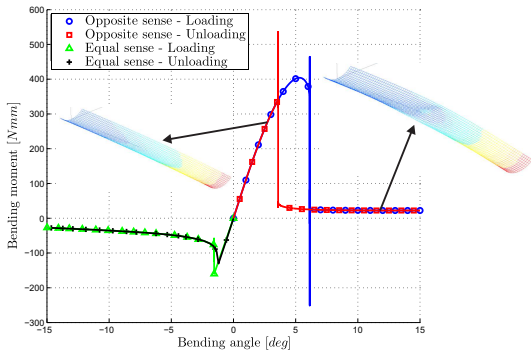
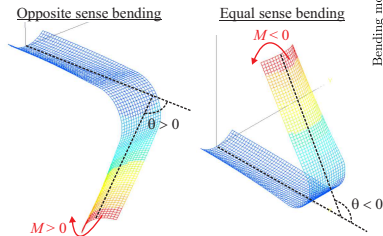
- ▶ Elastic energy
- ▶ Structural deformation
- ▶ No external energy sources
- ▶ Cheap, simple, reliable
- ▶ Space applications



*S. Hoffait et al.*

# INTRODUCTION - TAPE SPRINGS

- ▶ Highly nonlinear
- ▶ Buckling, hysteresis and self-locking
- ▶ Senses of bending



# INTRODUCTION - TYPES OF DAMPING

## Structural damping:

- ▶ Property of the material
- ▶ Simple rheological models: Maxwell, Kelvin-Voigt, ...
- ▶ Advanced models: Prony series, Rayleigh damping, ...

## Numerical damping:

- ▶ Property of the solver
- ▶ Examples: Newmark, HHT, generalized- $\alpha$ , Runge Kutta, ...
- ▶ Role: convergence, filter spurious modes, ...

# OBJECTIVES

## State of the art:

- ▶ In the majority of the previous works, F. E. analyses with numerical damping
- ▶ Structural damping rarely represented
- ▶ Notable exceptions: Kwok & Pellegrino (2011) and Mobrem & Adams (2009)

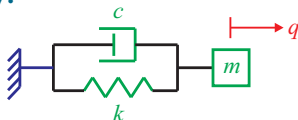
## Objectives:

- ▶ Determine the impact of the two types of damping
- ▶ Introduce some structural damping
- ▶ Reduce the dependence to numerical damping

Simulation without structural damping (*Hoffait et al.*).

# ONE DEGREE OF FREEDOM SYSTEM

## Case study:



$$\omega^2 = \frac{k}{m} \quad \varepsilon = \frac{c}{2\omega m}$$

## Equation of motion:

$$\ddot{q}_{n+1} + 2\varepsilon\omega\dot{q}_{n+1} + \omega^2 q_{n+1} = 0$$

**System to be solved:** (with the update formulae of the solver)

$$\mathbf{E}(\omega h, \varepsilon) \mathbf{x}_{n+1} = \mathbf{B}(\omega h, \varepsilon) \mathbf{x}_n$$

## Amplification matrix:

$$\mathbf{A}(\omega h, \varepsilon) = \mathbf{E}(\omega h, \varepsilon)^{-1} \mathbf{B}(\omega h, \varepsilon)$$

# ONE DEGREE OF FREEDOM SYSTEM

## Spectral radius:

$$\rho(\omega h, \varepsilon) = \max(|\lambda_1|, |\lambda_2|, |\lambda_3|)$$

to assess the level of dissipation in the model.

## For a valid numerical solution:

Low frequencies $\omega h \lesssim 0.5$	High frequencies $\omega h \gtrsim 2$
Accuracy	Convergence
Good representation of the physical behaviour	Filtering of high frequency modes
Good approximation of the real damping	

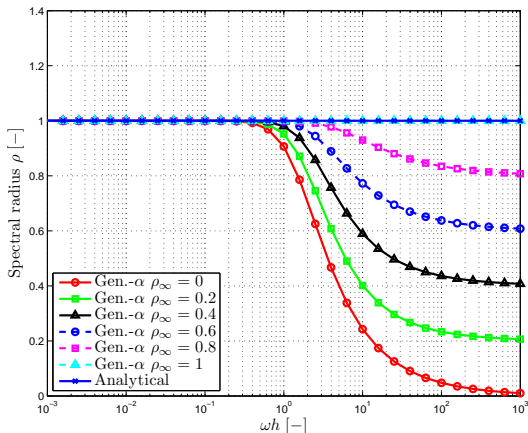


# ONE DEGREE OF FREEDOM SYSTEM

## Structural damping and numerical damping:

$$\varepsilon = 0$$

$$0 \leq \rho_{\infty} \leq 1$$

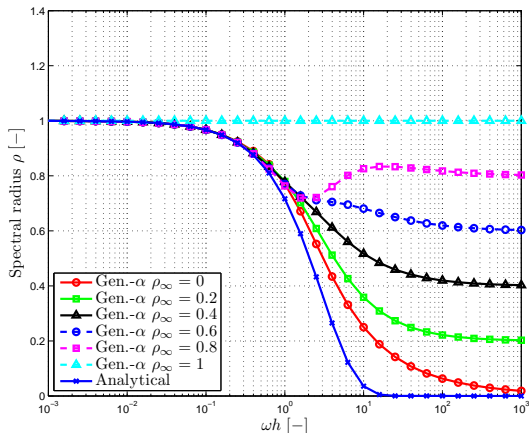


# ONE DEGREE OF FREEDOM SYSTEM

## Structural damping and numerical damping:

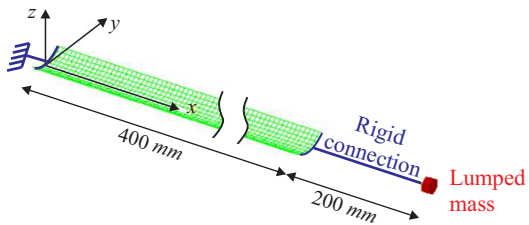
$$\varepsilon = 0.33$$

$$0 \leq \rho_{\infty} \leq 1$$



# TAPE SPRING - DYNAMIC ANALYSIS

## Case study:

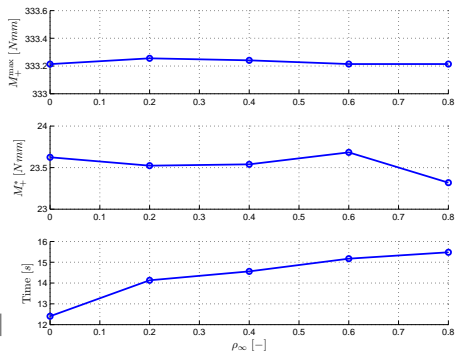
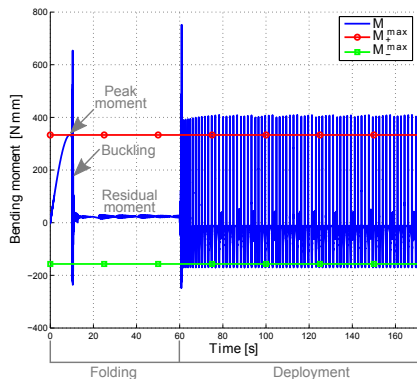


**Folding:** in the opposite sense with a bending angle of  $60^\circ$

**Deployment:** dynamic analysis for 110 s

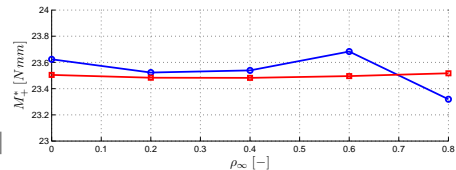
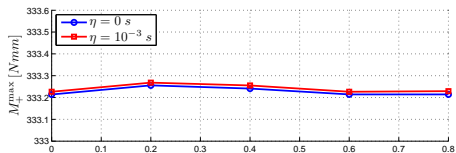
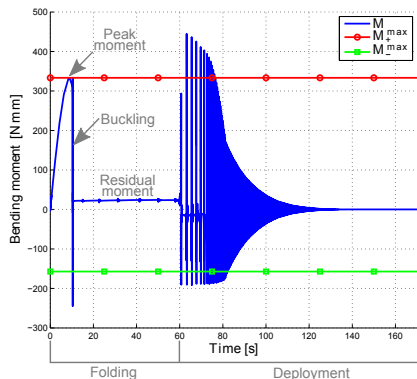
# TAPE SPRING - DYNAMIC ANALYSIS

Without structural damping:



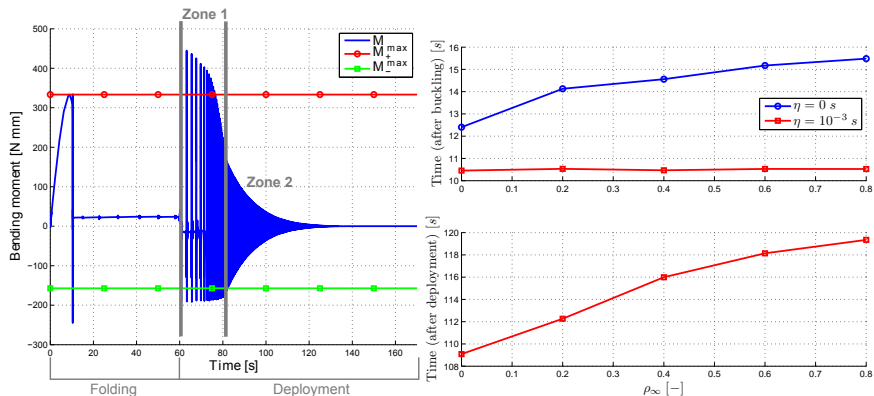
# TAPE SPRING - DYNAMIC ANALYSIS

With structural damping:



# TAPE SPRING - DYNAMIC ANALYSIS

With structural damping:



# TAPE SPRING - DYNAMIC ANALYSIS

## Comparison of the displacements:

With structural damping

Without structural damping

# CONCLUSIONS

- ▶ The two types of damping are required for a valid numerical solution
- ▶ Adding some structural damping:
  - ▶ reduces the dependence to numerical damping
  - ▶ ensures a correct representation of the damping of the oscillations after deployment
  - ▶ permits to model the self-locking phenomenon



# THANK YOU FOR YOUR ATTENTION

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